## Town of Bar Nunn

# Antelope Drive and Salt Creek Highway Access Study and Plan 

FINAL Report

March 2021


Prepared For:


CASPERAREA
METROPOLITAN PLANNING ORGANIZATIDN
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## 1. Introduction

## Project Purpose

The Casper Area MPO, to forward the goals and projects outlined in the latest version of their Long-Range Transportation Plan Connecting Crossroads, sought to draft a traffic study and plan for the Town of Bar Nunn that would examine the feasibility and long-term effects of abandoning a costly section of Antelope Drive. The Town of Bar Nunn continues to experience residential and industrial growth. Since the opening of the new I-25 interchange at Westwinds Road, travel patterns to and from Bar Nunn have changed. Most of Salt Creek Highway, particularly the section directly adjacent to its intersection with Antelope Drive in Bar Nunn, was not designed to safely and sustainably handle current or future traffic volumes or truck traffic, nor is the corridor or the surrounding area designed to accommodate pedestrians or bicycles.

A pavement study has determined that the section of Antelope Drive near Salt Creek Highway, due to the poor alkali soils, has significant structural and pavement deficiencies that necessitates a complete reconstruction. The Bar Nunn Town Council wanted to evaluate the effects of funding improvements to the other access points into the town and permanently closing and abandoning the damaged section of Antelope Drive, should they choose to do that.

The Town and the Casper Area MPO have commissioned the Town of Bar Nunn Antelope Drive and Salt Creek Highway Access Study and Plan to assess traffic and safety conditions, examine access to/from Bar Nunn, and identify short and mid-term improvements to Salt Creek Highway. The study evaluated:

- Effects of potentially abandoning and removing a costly section of Antelope Drive (currently the most heavily utilized access point into the town of Bar Nunn) due to failing subsurface conditions and excessive maintenance costs.
- Modifying traffic control devices (e.g. all-way stop control or roundabout control) at one or more existing intersections along Salt Creek Highway, providing enhanced access to the Town as well as traffic calming, traffic safety improvements and enhanced aesthetics. The ability to provide a "town gateway" could be included in these alternatives.
- Reimagining Sunset Boulevard as a possible "Main Street" and foster the creation of an enhanced streetscape and civic atmosphere around Sunset Boulevard, directly adjacent to the plot of land selected for the construction of a new Bar Nunn Town Hall.
- Re-directing and encouraging residential and commercial traffic currently utilizing Antelope Drive to the new interchange at Westwinds Road and alternative access points into town to reduce truck impacts on Salt Creek Highway and residential streets.
- Feasibility of an additional access to Salt Creek Highway from Wardwell Industrial Avenue to reduce or eliminate truck traffic in residential areas.
- Potential road improvements along Salt Creek Highway, including pedestrian, bicycle and bus facilities. Possible pedestrian and bicycle connections into Bar Nunn were also considered.

This report documents current traffic volumes, evaluates safety and operations within the study corridor, develops land use and traffic forecasts, evaluates future traffic conditions, identifies improvements for Salt Creek Highway and local roadways to provide acceptable traffic operations, and develops conceptual designs and cost estimates. Lastly, the report presents a recommendation and timeline for implementing roadway improvements, and identifies next steps (e.g., lead actors, funding and permitting) to finalize a Corridor Plan that serves all stakeholders. Overall, this plan will serve as a policy, management, and planning guide for future corridor management, access changes, improvement of multi-modal transportation infrastructure, and the development of adjacent land along Salt Creek from Howard Street to Westwinds Road.

## History

| Study Name | Year | Key Findings |
| :--- | :--- | :--- |
| Casper Area MPO <br> Wayfinding Master <br> Plan | 2020 | This document has been prepared to provide guidance for the <br> implementation of wayfinding directional signage within the Casper <br> Metropolitan Region. The report provides strategic, logical, and <br> aesthetical approaches to orient and direct visitors and residents to and <br> around key area destinations. The wayfinding signage will contribute to an <br> overall "sense of place," serving as an extension of the community's brand <br> and embellishing the key places that residents hold in high regard. |
| Town of Bar Nunn <br> - Comprehensive <br> Plan | 2018 | "Building Bar Nunn" identified areas within the transportation network that <br> offer challenges to transportation in Bar Nunn plus opportunities to <br> improve transportation and quality of life. As the population of Bar Nunn <br> grows and with a high percentage of residents traveling to adjacent <br> communities to go to work, it is increasingly important that roadway, <br> pedestrian and bicycle connections between communities be improved <br> and expanded. As adjacent communities update their Major Street Plans <br> to address future growth and congestion, Bar Nunn has the opportunity to <br> connect and expand on those proposed systems in an effort to craft and <br> define the character and development of Bar Nunn over the next 10 years. |
| Town of Bar Nunn <br> - Transportation <br> Plan | 2017 | This document laid out a long-term transportation plan to guide citizens <br> and government officials of Bar Nunn and help them to identify and <br> evaluate existing barriers to transportation, travel patterns as well as street <br> and bike/pedestrian networks. Further, the plan outlines opportunities and <br> the relevant strategies to achieve improvements in connectivity, functional <br> classification, land use, urban form, block configuration, roadway spacing, <br> block size, access management, and transit. |
| Polaris Drive/ <br> Westside Blvd. <br> Planning and <br> Environmental <br> Linkages Study | 2014 | This study identified transportation needs and developed alternative <br> solutions, with a focus on construction of a new Polaris Drive arterial road <br> as a bypass to Salt Creek Highway. It developed a Purpose and Needs <br> Statement, analyzed traffic and safety conditions, determined existing and <br> future needs, developed and evaluated alternatives, performed alternative <br> screening, evaluated possible environmental impacts, conducted agency <br> and public involvement, and prepared the final report. |


| Casper Area <br> Trails, Path and <br> Bikeway Plan | 2013 | The Plan builds upon previous planning and ongoing infrastructure <br> implementation efforts to identify gaps and barriers to walking and biking <br> in the Casper Area, and recommends both infrastructural improvements, <br> as well as policies, programs, and practices that support these modes. <br> The Bar Nunn area was included in this plan. |
| :--- | :--- | :--- |
| Bar Nunn I-25 <br> Interchange <br> Feasibility Study | 2013 | Prepared for WYDOT, this study assessed the applicability of a new <br> interchange located somewhere along I-25 between Westwinds Road and <br> McMurry Blvd. An interchange at Westwinds Road was identified as the <br> preferred alternative. |
| Bar Nunn Salt <br> Creek Intersection <br> and Subarea <br> Planning Traffic <br> Study | 2012 | Study to identify needs and recommend potential cost-effective <br> improvements to Salt Creek Highway from the intersection of Antelope Dr. <br> to the intersection of McMurry Blvd. Also included in this study is <br> development of interim and final street network configurations that will <br> accommodate growth as it occurs. This Study assisted the MPO in making <br> decisions regarding roadway and intersection improvements and/or <br> defining areas that may need additional study and public input |
| Bar Nunn Traffic <br> Study | 2010 | Initial study to examine the effects of new development due to energy <br> production ramping up. The study developed a list of transportation needs <br> and recommended possible solutions. |
| Salt Creek <br> Highway/ McMurry <br> Boulevard <br> Corridor Study | 2008 | This study examined existing and future transportation needs along Salt <br> Creek Highway in Bar Nunn. Future planned development was taken into <br> account. This study laid the foundation for the new l-25 Westwinds <br> interchange. |
| Bar Nunn <br> Community <br> Development Plan | 2008 | This plan guides future land development in Bar Nunn. |

## Goals and Objectives

The goals of the project include:

- Improve quality of life for Bar Nunn residents
- Improve road and intersection safety
- Make Bar Nunn more attractive for future development by improving Town access, visibility, and aesthetics
- Minimize heavy truck impacts to roads and town residents
- Make Bar Nunn accessible for all modes of transportation

The project objectives are:

- Provide opportunities for the residents, business owners and key stakeholders to give input to inform the study
- Develop a list of possible existing and future transportation needs in the study area
- Develop and evaluate road, intersection and traffic control improvement alternatives
- Examine possible multi-modal improvements and connections to Bar Nunn
- Develop an improvement plan for roads and intersections in and around Bar Nunn, that can be implemented with identified available funding
- Develop a final report that summarizes study data, procedures and recommendations


## 2. Existing Conditions

## Study Area

The study area (Figure 1) includes Salt Creek Highway from Westwinds Road to Howard Street, including the intersections of McMurray Blvd, Prairie Lane, Sunset Blvd, Antelope Drive and Howard Street. As part of the construction of the new Westwinds Road interchange, the northern end of Salt Creek Highway was realigned and improved to tie into Westwinds Road further west of the new I25 ramps. Roads leading into Bar Nunn were also evaluated in this study.


FIGURE 1 -- STUDY AREA MAP

## Existing Infrastructure

Roadway Geometry and Drainage: Salt Creek Highway is a two-lane undivided roadway with a posted speed limit of 40 MPH that ranges from 22 feet to 46 feet wide. There are no existing paved shoulders, sidewalks or bicycle facilities. Within the study area, Salt Creek Highway is an open-section roadway, with runoff directed into shallow ditches on both sides of the pavement. The exception is the approximately 4,550 -foot-long segment between Westwinds Road and a point approximately 450 feet north of McMurry Boulevard, where the roadway has been improved to provide three lanes (one lane in each direction and a center turn lane), plus concrete curb and gutter to convey storm runoff. All side streets that intersect Salt Creek Highway within the study area are open section roadways as well. Currently, the Wyoming Department of Transportation (WYDOT) owns and maintains Salt Creek. However, both Bar Nunn and the MPO anticipated that WYDOT will transfer ownership of the road to the Town of Bar Nunn in 2021 as is without any further improvements.

Intersection and Traffic Controls: Salt Creek Highway originates at an orthogonal "T" intersection at Westwinds Road, then proceeds south to intersect McMurry Boulevard, Coyote Avenue, Prairie Lane, Sunset Boulevard, and Antelope Drive at " T " intersections, with the side streets at angles of approximately 80, 90, 75, 80, and 75 degrees, respectively. Finally, the Salt Creek Drive/Howard Street intersection is a fourway orthogonal intersection. Intersection lighting is provided on utility poles at most study intersections. Six of the seven intersections (Westwinds Road, McMurry Blvd., Coyote Avenue, Prairie Lane, Sunset Blvd., and Antelope Drive) are controlled by stop signs on the minor street, while an all-way stop now controls the Salt Creek Highway/Howard Street intersection.

Pavement Condition: Of note is the ongoing deterioration of Antelope Drive just west of Salt Creek. Severe spring heaving and fall settling, water seepage and rutting of the roadway are evident.


FIGURE 2 -- PAVEMENT DETERIORATION ON ANTELOPE DRIVE

Directional/ Wayfinding Signage: Westwinds Road provides all access into the study area from the north, collecting traffic from both I-25 and points north along Salt Creek Highway, which continues in a separate segment east of $\mathrm{I}-25$, connected to the study area segment by Westwinds Road.

South of Bar Nunn, Howard Street provides access from I-25, while Salt Creek Highway continues as Highway 254 into Mills. The exit 191 guide sign identifies the destination as "Wardwell Road/Bar Nunn" and specific services signs identifying the Loaf ' $n$ Jug convenience store and the KOA campground. Directional signs at the l-25 northbound and southbound ramps, as well as at the Howard Street/Salt Creek Highway intersection, guide motorists to Bar Nunn.

Within Bar Nunn, monument signs identify the entrances to Antelope Drive, Sunset Boulevard, and McMurry Boulevard as gateways to Bar Nunn.


FIGURE 3 -- BAR NUNN MONUMENT SIGN AT MCMURRY BLVD

Utilities and Environmental Features: Water lines extend along both sides of Salt Creek Highway between Howard Street and Antelope Drive, and along the western side of the roadway between Antelope Drive and Prairie Lane, and a short segment of sanitary sewer extends along the east side of Salt Creek Highway for about 870 feet south of Antelope Drive. There are utility poles carrying electric, telephone, and cable television on both sides of Salt Creek between Howard Street and Antelope Drive and along the west side of the roadway between Antelope Drive and the relocated segment north of McMurry Boulevard. Where Salt Creek has been relocated west to accommodate the Westwinds Road interchange, the utility poles have maintained their previous alignment.

Two study roadways (Antelope Drive and Salt Creek Highway) are bisected by a salt flat that consists of alkali soils, with a shallow water table and upward gradient from a confined aquifer. The soft, saturated and expansive clay and low sulfate levels do not allow for natural drainage of water during storm or melt events. This leads to severe spring heaving and fall settling of pavement, water seeping up through the roadway, and rutting and pavement damaged due to the soft subgrade. These signs of pavement deterioration have been visible for the past few years on Antelope Drive but have not yet occurred along Salt Creek Highway in the vicinity of the salt flat. Stabilizing the roadway would require expensive retrofits (estimated at $\$ 500,000$ for Antelope Drive and $\$ 4,000,000$ for Salt Creek Highway) to install underdrains and parallel French drains, along with reconstruction of surface, base, subbase and subgrade with lime.

Salt Creek Highway through Bar Nunn does not intersect any floodplains, steep slopes, critical habitats, or other environmentally sensitive areas.

## Traffic Volumes

Current and historical traffic volume data was obtained from the Casper Area MPO's traffic count database and traffic monitoring program. Map 1 displays typical daily traffic volumes throughout the study area. The highest-volume road segment in the study area is Salt Creek Highway between Howard Street and Antelope Drive, with volumes fluctuating between approximately 5,600 and 7,900 vehicles per day over the last five years. Traffic volumes gradually decrease northwards within the study area, dropping by about 2,500 at Antelope Drive, about 500 at Sunset Boulevard, about a further 500 at Prairie Lane, and a further 2,000 at McMurry Boulevard, to average under 500 north of McMurry Boulevard. Other than Salt Creek Highway, Antelope Drive is the highest-volume roadway within Bar Nunn, serving approximately 2,400 trips per day, followed by McMurry Boulevard, which serves just under 1,800 trips per day.

In the Casper region, as across the nation, traffic volumes dipped significantly in Spring 2020 due to COVID19; monthly traffic volumes in the area were approximately 30 percent lower in April than at the same time in previous years. Traffic volumes rapidly rebounded, however, and by August were less than ten percent lower in the region than in August of previous years. Therefore, while the pandemic temporarily altered travel patterns and has introduced greater uncertainty in future travel demand, it has not fundamentally changed how many people travel by car in the Casper area.

The most significant change in most recent years that has affected traffic volumes in Bar Nunn has been the opening of the Westwinds Road interchange with I-25 (Exit 194). 24-hour counts collected in 2020, after the opening of the interchange in 2018, suggest that traffic volumes on Salt Creek Highway, Antelope Drive, and McMurry Boulevard have declined modestly since the opening of the Westwinds interchange, as shown in Map 1. In contrast, traffic volumes-especially of heavy vehicles-have increased on Westwinds Road, illustrating that some freight traffic has adapted and is now using the new interchange to access the industrial areas north of the Town. Because traffic counts for the study were collected during the Fall of 2020 and were not adjusted for seasonal factors, the variability in traffic patterns may be attributed to both the COVID pandemic impact on remote work and school conditions as well as from the typical variability in seasonal traffic volumes. The modest decline in daily traffic volumes along portions of Salt Creek, Antelope and McMurray also indicate that some traffic from the Howard Street interchange has shifted to the Westwinds interchange.


## Crash Assessment

Historical crash data from 2004-2014 and 2018-2019 was obtained from the Casper Area MPO and analyzed for this report. Most crashes occurred at the intersections of Salt Creek Highway at Howard Street, and Salt Creek Highway at Antelope Drive, as shown in Map 2. The Howard Street intersection averages approximately two to three reported crashes per year, while the Antelope Drive intersection averages approximately one reported crash per year and a further one reported crash per year takes place along Salt Creek Highway between Howard and Antelope. There are no crash clusters along Salt Creek Highway north of Antelope Drive.

At Howard Street (see Table 14 in Appendix A), a majority of the 26 reported collisions were angle crashes with smaller numbers of rear-end and sideswipe collisions, and a majority took place during daylight in clear conditions. Since the intersection was converted to all-way stop control in 2015, no crashes were reported in 2018 or 2019.

At Antelope Drive (see Table 14 in Appendix A), angle crashes constitute at least half of 15 reported collisions ( 30 percent of Antelope/Salt Creek crash reports did not indicate a crash type), and a majority took place during clear and dry conditions. Only two crashes since 2004 at the Salt Creek/Antelope intersection have resulted in any injury, and one crash resulted in a fatality.

Elsewhere in the study area (see Table 15 in Appendix A.), the largest proportion of crashes were singlevehicle crashes (about 30 percent of crashes) and rear-end collisions (about 20 percent). Notable distinctions between the crash patterns at Antelope Drive, Howard Street, and elsewhere include the following:

- A larger proportion of collisions took place during snow or when the road was ice-covered at Antelope ( 40 percent of all crashes) than at Howard (10 percent) or elsewhere in the study area (33 percent of all crashes).
- A larger proportion of collisions resulted in possible injury or worse outside Antelope and Howard Street intersections ( 33 percent) than at Antelope ( 20 percent) or Howard ( 23 percent)
- Unlike at the Antelope, Sunset, Prairie, and McMurry intersections, there is no lighting in the vicinity of the Howard Street intersection. Accordingly, five crashes were reported to have taken place in dark, unlighted conditions there and no crashes were reported to have taken place at any other intersection in dark, unlighted conditions. 13 crashes did take place in dark, unlighted conditions at non-intersection locations within the study area.

Detailed crash information may be found in Appendix A.


## Travel Times

Map 3 shows estimated peak hour travel times for selected routes within the study area. These travel times are based on TomTom traffic data validated by field-measured travel time runs conducted in October 2020 during peak commuting hours. The travel time analysis assessed three origin-destination pairs: the McMurry/Antelope intersection and I-25 south of Howard Street, the Trails End/Sunset intersection and I25 south of Howard, and the Sunset/Antelope intersection and the Salt Creek/Antelope intersection. Table 1, below, summarizes the results.

TABLE 1. TRAVEL TIME RESULTS

| McMurry/Antelope to <br> l-25 south of Howard | via Antelope and Salt Creek | via McMurry, Salt Creek, <br> Westwinds, and l-25 | Difference |
| :--- | :---: | :---: | :---: |
| Southbound | $5: 20$ | $7: 00$ | $1: 40$ |
| Northbound | $5: 30$ | $7: 20$ | $1: 50$ |


| Trails End/Sunset to <br> $\mathrm{I}-25$ south of Howard | via Antelope and Salt Creek | via Sunset and Salt Creek | Difference |
| :--- | :---: | :---: | :---: |
| Southbound | $5: 20$ | $6: 30$ | $1: 10$ |
| Northbound | $5: 40$ | $6: 50$ | $1: 10$ |


| Sunset/Antelope to <br> Salt Creek/Antelope | Local Route A via Antelope | Local Route B via Sunset <br> and Salt Creek | Difference |
| :--- | :---: | :---: | :---: |
| Southbound | $1: 00$ | $1: 30$ | $0: 30$ |
| Northbound | $1: 00$ | $1: 20$ | $0: 20$ |

As the data shows, travelling via McMurry, Salt Creek, Westwinds, and I-25 adds at least one minute forty seconds to travel time via Antelope and Salt Creek from McMurry/Antelope to I-25 south of Howard and vice versa. Travelling via Sunset adds about one minute ten seconds to travel time via Antelope from the Trails End/Sunset intersection to I-25 south of Howard and vice versa. Finally, the short trip between Sunset/Antelope and Salt Creek/Antelope takes twenty to thirty seconds longer via Sunset and Salt Creek than via Antelope. Note that there are worst case travel time changes, for motorists going to or from the last house on Antelope near Salt Creek Highway. Vehicles traveling to or from houses closer to Palomino and Sunset would experience less significant travel time increases if Antelope Drive is closed at Salt Creek Highway.


MAP 3 - ESTIMATED TRAVEL TIMES FOR SELECTED ROUTES

## Capacity Analysis

A capacity analysis of the Salt Creek Highway corridor was performed using Synchro 10, a macroscopic and deterministic traffic analysis software which implements the Highway Capacity Manual (HCM) methodology. Existing and historical 2013 peak hour PM intersection traffic volumes along with percent heavy vehicles are shown in Map 4. Truck, bus and motorhome traffic consists of between $3 \%$ to $12 \%$ of the overall traffic volumes. The existing AM and PM peak hour traffic volumes, intersection traffic control type, and geometric data such as number of lanes, lane configuration, and storage length were input into Synchro to create a base conditions traffic model.

Synchro was used to report the delay, Level of Service (LOS), and volume-to-capacity ratio at the six study intersections. The analysis results are presented in Table 2. Detailed Synchro reports are available in Appendix C. The existing conditions analysis shows that all intersections operate acceptably and that there are no operational concerns along the study corridor. All movements are below capacity and operate at an LOS C or better.

TABLE 2 -- SALT CREEK HIGHWAY CAPACITY ANALYSIS - EXISTING CONDITIONS

| \# | Intersection | Approach* | Existing Conditions - AM (PM) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{aligned} & \text { Delay/Veh } \\ & \text { (sec) } \\ & \hline \end{aligned}$ |  | Level of Service |  | Volume/ Capacity Ratio |  |
| 1 | Salt Creek Hwy \& Westwinds Rd | Westbound | 3 | (1) | A | (A) | 0.01 | (0.01) |
|  |  | Northbound | 9 | (9) | A | (A) | 0.02 | (0.01) |
| 2 | Salt Creek Hwy \& McMurry Blvd | Eastbound | 9 | (9) | A | (A) | 0.18 | (0.09) |
|  |  | Northbound | 7 | (7) | A | (A) | 0.03 | (0.08) |
| 3 | Salt Creek Hwy \& Prairie Ln | Eastbound | 9 | (9) | A | (A) | 0.03 | (0.03) |
|  |  | Northbound | 2 | (1) | A | (A) | 0.02 | (0.02) |
| 4 | Salt Creek Hwy \& Sunset Blvd | Eastbound | 10 | (9) | A | (A) | 0.04 | (0.01) |
|  |  | Northbound | 1 | (1) | A | (A) | 0.01 | (0.03) |
| 5 | Salt Creek Hwy \& Antelope Dr | Eastbound | 11 | (10) | B | (A) | 0.24 | (0.11) |
|  |  | Northbound | 4 | (4) | A | (A) | 0.07 | (0.12) |
| 6 | Salt Creek Hwy \& Howard St | Eastbound | 10 | (10) | B | (B) | 0.06 | (0.03) |
|  |  | Westbound | 11 | (12) | B | (B) | 0.53 | (0.39) |
|  |  | Northbound | 12 | (15) | B | (B) | 0.36 | (0.51) |
|  |  | Southbound | 13 | (12) | B | (B) | 0.72 | (0.35) |

*Note: Approaches with free movements and no vehicle delay are omitted


MAP 4 - INTERSECTION TRAFFIC VOLUMES

## Multi-Modal Assessment

Many Bar Nunn residents walk and bike on a regular basis (mostly during warmer months), in addition to driving, both for local recreation and to in-town destinations such as parks and mailboxes. As shown in Map 5 , there are significant sidewalk networks within the residential neighborhoods of Bar Nunn, but these networks are disconnected from the commercial center of the Town, where government buildings, restaurants and recreational sites are located, and from Salt Creek Highway. While there are no dedicated bicycle facilities in Bar Nunn, streets are wide and volumes are low, so bicycling is still a safe option for many riders. There are no transit stops within Bar Nunn, but middle and high school students do board school buses at multiple locations along Antelope Drive, and students of all grade levels board buses along Nez Perce Trail near High Plains Park.


MAP 5 - SIDEWALKS AND PEDESTRIAN DESTINATIONS

## Existing Land Use and Zoning

Existing land uses in Bar Nunn comprise a commercial and industrial core roughly bounded by Bel Vista Drive, Antelope Drive, Salt Creek Highway, and McMurry Boulevard and multiple, mostly single-family, residential neighborhoods to the west and north of the industrial core. As shown in Map 6, the Town anticipates further industrial development along Westwinds Road and relocated Salt Creek Highway and expanded residential neighborhoods through the rest of the Town.


MAP 6 - EXISTING LAND USE

## 3. Public Involvement

To solicit community and stakeholder input, a public open house was held on October 14, 2020 at The Hangar from 5 PM to 8 PM. Meeting announcements were posted on the MPO and Town newsletter and in social media and website channels. A questionnaire survey was also developed to solicit input on existing conditions. The survey was available online on the MPO and Town of Bar Nunn website, as well as hard copies at Bar Nunn Town Hall. The meeting included a short presentation on the study purpose and goals and objectives, plus existing data and conditions, followed by an open house with roll plans and posters of the study area and transportation data. Approximately 20 people attended. The attendees were asked to provide feedback on the existing conditions findings and offer ideas on improving access to the Town.


Public Engagement at Public Meeting \#1

## Public Meeting Comments / Survey Results

The questionnaire survey asked for opinions on traffic safety, congestion, travel routes and multimodal needs along Salt Creek Highway and within the Town of Bar Nunn. The hard copy of the survey is shown in Figure 4.


FIGURE 4 - COMMUNITY TRANSPORTATION SURVEY

The results of the community survey, highlighted in Figure 5, confirmed that Town residents are nearly evenly split between accessing l-25 via the Westwinds interchange and the Howard Street interchange. The intersection with the highest level of safety concern and perceived traffic delay was Antelope Drive and Salt Creek Highway. No strong preference on a preferred traffic control type was expressed.

Additional comments received at the public meeting included the traffic delay impacts of school bus stops/ routes, traffic safety for left-turning traffic along Salt Creek Highway and the need for separate left-turn lanes, RV parking issues along Sunset Blvd and truck traffic on residential streets from the Wardell industrial area.

A second public open house took place on January $14^{\text {th }}, 2021$, at The Hangar from 4 PM to 7 PM, to present and discuss alternative solutions developed for this study. This meeting and resulting input is described later in this report.


FIGURE 5 -- COMMUNITY SURVEY RESULTS

## Agency Input - Town input

The Town of Bar Nunn's Mayor and maintenance staff provided input during the stakeholder and community engagement process. Due to the Town's street network following the old airport runways and taxiways, the Town is burdened with excessive roadway widths and pavement, plus close parallel roads that both increase maintenance costs and encourage high travel speeds within the Town. With the pending transfer of Salt Creek Highway to the Town's roadway network, the need to manage future roadway maintenance costs is critical. The Town expressed concerns about the sustainability of maintaining Antelope Drive, with costs for subsurface reconstruction and underdrains potentially exceeding \$500,000. The Town has also begun 'tactical' traffic calming designs including chokers and raised crosswalks along McMurray Blvd and Antelope Drive to both slow traffic down and reduce truck traffic on residential streets.

## 4. Existing Needs Assessment

## Needs and Barriers

The existing conditions assessment identified several needs for, and barriers to safe multi-modal transportation along the Salt Creek Highway corridor, and access to the Town including:

- Need to prioritize short-term improvements, whereas previous study recommendations focused primarily on long-term transportation improvements.
- Need to reduce roadway maintenance costs for the Town due to excessive pavement widths and pavement failures. Designs to reduce pavement width and roadway mileage are needed.
- Need to reduce excessive speeds on Town roads, due mainly to the straight, wide roads.
- Need to improve the visibility of the Town. There is a lack of a formal gateway and wayfinding signage that are visible and command attention for the Town along Salt Creek Highway. Enhanced streetscapes and gateway features are needed.
- Need to improve traffic safety. There is a history of frequent crashes at Salt Creek and Antelope and at the intersection of Salt Creek and Howard. Upgraded traffic controls are needed.
- Need to improve multi-modal connections. There is a lack of pedestrian and bicycle connections along Salt Creek Highway south of Town to the existing commercial area.
- The current slow growth in the energy industry and the lack of diversity in the economy are barriers.


## Opportunities and Constraints

The existing conditions assessment identified several opportunities and constraints to address when developing future short- and long-term alternatives, including:

- Opportunity to reduce speeding by development of low-cost traffic calming improvements.
- Opportunity to enhance safety by upgrading intersection traffic controls and accommodating all design vehicles and traffic diversions from Antelope Drive.
- Opportunity to improve the town visibility and aesthetics by developing a streetscape plan for the primary Town access point that includes a gateway feature/ pocket park.
- Opportunity to reduce truck impacts on residential streets with construction of a new direct roadway connection from Wardell Road to Salt Creek Highway, with minimal right-of-way acquisition.
- Opportunity to enhance pedestrian and bicycle access to Bar Nunn by developing a multi-modal connection from the Town to the commercial area along Howard Street within the existing right-ofway and minimizing utility impacts.
- Opportunity to ensure that future improvements of Salt Creek Highway can handle planned growth.
- Available right-of way and funding are constraints.


## 5. Alternatives Development

The purpose of the Town of Bar Nunn Antelope Drive and Salt Creek Highway Access Study and Plan is to assess traffic and safety conditions, examine access to/from Bar Nunn, and identify short and long-term improvements to Salt Creek Highway. The goals of the alternatives are to 1) meet identified needs, 2) be cost-effective, 3) be feasible and implementable, and 4) improve quality of life for Town residents.

Alternative development was based on input from the MPO, Town Staff, Town Mayor and the public. The alternatives developed and evaluated for this report include:

- Street Pattern Changes: Developing ways to accommodate the possible abandoning and removing a costly section of Antelope Drive (currently the most heavily utilized access point into the town of Bar Nunn) due to failing subsurface conditions and excessive maintenance costs.
- Traffic Control Changes: Modifying traffic control devices (e.g. all-way stop control or roundabout control) at one or more existing intersections along Salt Creek providing enhanced access to the Town to provide traffic calming, improve traffic safety and enhance aesthetics. The ability to provide a "town gateway" could be included in these alternatives.
- Streetscape Improvements: Reconstructing Sunset Boulevard with an enhanced streetscape design from Antelope Drive to Salt Creek Highway that accommodates all modes of transportation.
- Wayfinding: Re-directing and encouraging residential and commercial traffic that currently use Antelope Drive to instead take other/better routes by employing better signage to the new interchange at Westwinds Road and alternative access points into town.
- New Roadways: Constructing a new access roadway to Salt Creek Highway from Wardwell Industrial Avenue to reduce or eliminate truck traffic in residential areas.
- Existing Roadway Improvements: Improving the typical cross-section along Salt Creek Highway to include separate left-turn lanes, a continuous two-way center turn lane, wider lanes, improved drainage, and/or a shared use side path for walking and biking.

The alternatives presented are intended to be considered for short-term (e.g., 1-2 years) or mid-term (3-5 years) implementation so as not to be duplicative of other long-term recommendations from other previous studies. These studies have developed unique long-term (e.g. town buildout) recommendations for the Town's future roadway network. Concept design plans are included in Appendix B.

## Short-term Alternatives

## (1) Concept \#0a - Full Closure of Antelope Drive

This alternative would remove all existing pavement between Salt Creek Highway and the former Arena Street intersection and divert all traffic from that segment to Sunset Boulevard. The intended goal of this concept is to relieve the Town of the drainage challenges and high maintenance requirements of Antelope Drive that stem from the poor soils and topography upon which the road lies. In addition, there have been more traffic crashes in recent years at the intersection of Antelope Drive and Salt Creek Highway than any intersection in the study area except for Howard Street at Salt Creek Highway (due to the skewed alignment), and this concept would remove that intersection.


FIGURE 6-CONCEPT \#OA - FULL CLOSURE OF ANTELOPE DRIVE

## (2) Concept \#0b - Partial Closure of Antelope Drive

This concept would limit traffic to eastbound-only between Salt Creek Highway and the former Arena Street intersection, divert westbound traffic from that segment to Sunset Boulevard, and eliminate all left turns at the Salt Creek Highway/Antelope Drive intersection. Once those restrictions are in place, approximately half of Antelope Drive's pavement width could be removed. The resulting roadway would be approximately 14 feet wide. Like Concept \#0a (full closure of Antelope Drive), the purpose of this concept is to reduce the drainage challenges and maintenance requirements of Antelope Drive by removing pavement and reduce the safety concerns present at the Salt Creek Highway and Antelope Drive intersection by removing left-
turn movements.


FIGURE 7 - CONCEPT \#OB - PARTIAL CLOSURE OF ANTELOPE DRIVE

## (3) Concept \#0c - Wayfinding and Gateway Signage

This concept would modify or replace guide signs along I-25 to sign Westwinds Road (Exit 194) as the Bar Nunn interchange exit and install a large Bar Nunn welcome monument sign, visible from I-25, at the Salt Creek Highway and Sunset Boulevard intersection. Highway guide signs would be designed according to the MUTCD, while the monument sign would be designed according to the Casper Area MPO's regional wayfinding system graphic design guidelines and fabrication recommendations. The goal of this concept is to direct motorists to Westwinds Road, rather than Howard Street, to access Bar Nunn. This would lower traffic volumes at the Salt Creek Highway/Howard Street intersection, lessen traffic's wear and tear on Antelope Drive, and draw motorists' attention to the Town as they pass on I-25.

## EXISTING SIGNAGE ON I-25



REVISED SIGNAGE ON I-25


FIGURE 8-CONCEPT \#OC - WAYFINDING SIGNAGE


FIGURE 9-CONCEPT \#OC - GATEWAY SIGNAGE

## (4) Concept \#1 - New left-turn lanes along Salt Creek Highway

This concept would add northbound left turn lanes on Salt Creek Highway at any or all of the intersections with Antelope Drive, Sunset Blvd, Prairie Lane, Coyote Avenue, and McMurry Blvd. One 12-foot travel lane would be maintained in each direction while and an 11-foot left turn lane could be added in the northbound direction at each selected intersection. Turn bays would be approximately 100 feet long and include appropriate tapers ranging in length from 100 feet to 240 feet. The purpose of this concept is to enhance traffic safety and the risk for rear-end crashes and reduce traffic delay at the intersections by separating left-turning from through traffic.


FIGURE 10-CONCEPT \#1 - NEW LEFT-TURN LANES ALONG SALT CREEK HIGHWAY,

## (5) Concept \#2 - Traffic control upgrades

This concept considers several traffic control upgrades including all-way stop, roundabout and signal control at multiple locations including Antelope Drive and Sunset Blvd. Initial assessment of traffic control needs, and locations suggests that signal control or all-way stop control would not be warranted based on existing or near-term traffic volumes. Locating upgraded traffic controls such as a roundabout at Antelope Drive would not serve to meet the project goals of reducing maintenance costs. Therefore, this concept would construct a new modern roundabout Sunset Boulevard, to accommodate trucks, buses and emergency vehicles. The roundabout's inscribed circle diameter (ICD) would be around 105 feet, the single lane circulating roadway would be 18 feet wide, it would have a 40-foot center island, and would include splitter islands, pedestrian ramps and crosswalks and an apron to accommodate Recreational Vehicles and Tractor Trailers. This concept has three objectives: reducing traffic delay at the Salt Creek Highway/Sunset Boulevard intersection, reducing turning crashes (currently at Antelope Drive) and providing a gateway treatment for Bar Nunn that establishes Sunset Boulevard as the primary entrance into the Town.


FIGURE 11 - CONCEPT \#2-ROUNDABOUT AT SUNSET
(6) Concept \#3 - Internal Town intersection improvements - Sunset / Antelope (channelization)

This concept outlines the installation of curb extensions, crosswalks, all-way stop signs and stop bars, and sidewalk or multi-use trail segments at the intersections of Antelope Drive with Sunset Boulevard and Prairie Lane. At each intersection, curb extensions would narrow each roadway to 24 feet wide, providing one 12foot travel lane in each direction. The multi-use trail would be 8 feet wide with a 2 - to 6 -foot buffer from the roadways, while the sidewalk would be 5 feet wide and immediately adjacent to Antelope Drive. This concept has multiple goals, including to improve drainage, reduce vehicle speeds, increase safety for pedestrians and bicyclists, and create an aesthetic and attractive streetscape at this central location within the Town.


FIGURE 12 -- CONCEPT \#3 - INTERNAL TOWN INTERSECTION IMPROVEMENTS - SUNSET / ANTELOPE

## (7) Concept \#4 - Streetscape Improvements Sunset from Salt Creek to Antelope

This concept presents the addition of wide decorative sidewalks, plantings, pedestrian lighting, street trees, and a pocket park along Sunset Boulevard between Salt Creek Highway and Antelope Drive. New curb along Sunset Boulevard would create a consistent roadway width of 28 feet to provide for one 14-foot travel lane in each direction. An 8-foot decorative multi-use path would be located on the north side of the roadway, with a 3.5 -foot landscaped buffer separating it from the roadway, while on the south side of the roadway, a similar path would be separated from the roadway by a 12 -foot landscaped buffer. The concept illustration shows an approximately 12,000 square foot pocket park about 210 feet east of Antelope Drive, but the park could be located at any location along this segment where right-of-way accommodates it. Similar to Concept \#3, the purpose of this concept is to create an attractive streetscape for Sunset Boulevard that is inviting to pedestrians and bicyclists and reflects its function as a gateway to the Town.


FIGURE 13 -- CONCEPT \#4-STREETSCAPE IMPROVEMENTS, SUNSET FROM SALT CREEK TO ANTELOPE


FIGURE 14 -- RENDERING OF SUNSET BOULEVARD STREETSCAPE IMPROVEMENTS

## Mid-term Alternatives

## (8) Concept \#5 - New location street connection to Salt Creek for truck access

Several options were considered for connecting the Wardell Industrial Area to Salt Creek, including a new location roadway and an extension of Coyote Avenue. The extension of Coyote Avenue would require additional horizontal curves and impact several parcels, so a new location roadway parallel to Coyote Avenue and McMurray Blvd was developed. This new street, approximately 1,635 feet long, between Salt Creek Highway and Wardwell Industrial Avenue, would be approximately 750 feet north of Coyote Lane and 1,370 feet south of McMurry Boulevard. The roadway would be constructed with a 28 -foot section to provide one 14-foot travel lane in each direction, with a stronger pavement section to accommodate heavy trucks. This concept would provide truck access to the industrial core of Bar Nunn directly from Salt Creek Highway, better separating motorists from freight traffic by providing access to Wardwell Industrial Avenue that does not rely on residential streets (McMurry Boulevard or Prairie Lane) where houses are located. It would also reduce the number of turns truck drivers need to make to access industrial properties along Wardwell Industrial Avenue from Salt Creek Highway.


FIGURE 15 - CONCEPT \#5 - NEW LOCATION STREET CONNECTION TO SALT CREEK FOR TRUCK ACCESS

## (9) Concept \#6 - Salt Creek cross-section improvements (center turn lane) with Shared Use Path (south of Antelope)

This concept would widen Salt Creek Highway to two lanes per direction between Howard Street and the widened section north of McMurry, essentially extending the widened section south through the Town, with left turn lanes at intersections and enclosed drainage/curbs. It would include a multi-use path along Salt Creek Highway from Howard Street to Antelope Drive. The widened roadway section would be a closed section with curb and gutter, have one 12-foot travel lane in each direction and a 12-foot two-way center turn lane that would allow for left-turns at all intersections. The pathway would be 10 feet wide and be separated from the roadway by a variable-width landscaped buffer. The intended goal of this concept is to reduce traffic delays and increase capacity, improve safety and drainage, and provide comfortable and safe accommodation for people walking and bicycling between Bar Nunn and points south.


FIGURE 16-CONCEPT \#6-CROSS-SECTION IMPROVEMENTS WITH SHARED USE PATH, TYPICAL SECTION


FIGURE 17-CONCEPT \#6-CROSS-SECTION IMPROVEMENTS WITH SHARED USE PATH, SHEET 1
(10) Concept \#7 - Shared Use path/sidewalk to Howard

This concept would construct a multi-use path along Salt Creek Highway from Howard Street to Antelope Drive but make no changes to the roadway itself. The pathway would be 10 feet wide and be separated from the roadway by a landscaped buffer ranging from 1 foot to 10 feet wide. This concept would provide comfortable and safe accommodation for people walking and bicycling between Bar Nunn and points south if the roadway improvements envisioned in Concept \#6 do not need to be made.


FIGURE 18 - CONCEPT \#7 - SHARED USE PATH/SIDEWALK TO HOWARD, TYPICAL SECTION


FIGURE 19-CONCEPT \#7-SHARED USE PATH/SIDEWALK ANTELOPE TO HOWARD

## 6. Future Land Use

## Travel Model Demographics and Land Use

This section describes the future growth rates and future conditions developed for this study. This effort focused primarily on two sources: The current and future land use (zoning) found in the draft Bar Nunn Future Land Use Plan maps (Town of Bar N999unn \& WLC Engineering, 2018); and the Casper Regional Travel Demand Model as updated for the Casper Area MPO (N. Marshall, February 2020). These were assessed and combined to derive the projected growth in 2025 land use and traffic volumes. An overview of each is provided below.

## Town Land Use Plan

In 2017, the Casper Area MPO funded an update to the Bar Nunn Future land use (and zoning) plan. While still in draft form the current land use and future zoning from this update (previous plans were adopted in 2008) were reviewed and used for the current study. Future land use plans are shown in Figure 20. As can be seen the parcels just to the west of I-25, east of Arena St. and adjacent (north and south) of Sunset Blvd are currently commercial properties. The proposed future land use expands the light industrial, commercial, and commercial highway zoning west of I-25 to extend from Antelope Drive in the south up to and beyond Westwinds Blvd to the north. Additional multi-family dwelling zoning has also been included in the undeveloped portions of the city primarily in the north. A summary of the future land use zoning is provided in Table 3. These were converted into potential future dwelling units and Commercial - Light Industrial square footage shown in Table 4.


FIGURE 20 -- TOWN OF BAR NUNN FUTURE ZONING (DRAFT)

TABLE 3 - FUTURE LAND USE SUMMARY

| Total Unbuilt | '000 sqft | Proportion |
| :--- | ---: | ---: |
| Residential - Low | 5,683 | $8 \%$ |
| Residential - Medium/High | 12,670 | $17 \%$ |
| Industrial | 41,062 | $56 \%$ |
| Commercial | 14,550 | $20 \%$ |
| Total | 73,965 | $100 \%$ |
|  |  |  |
| Westwind | $'^{\prime} 000$ sqft | Proportion |
| Industrial | 40,062 | $78 \%$ |
| Commercial | 11,422 | $22 \%$ |
| Total | 51,484 | $100 \%$ |
|  |  |  |
| McMurry | $\mathbf{\prime o 0 0} \mathbf{s q f t}$ | Proportion |
| Commercial | 3,351 | $26 \%$ |
| Residential | 9,583 | $74 \%$ |
| Total | 12,934 | $100 \%$ |
|  |  |  |
| Prairie | '000 sqft | Proportion |
| Commercial | 1,600 | $62 \%$ |
| Industrial | 1,000 | $38 \%$ |
| Total | 2,600 | $100 \%$ |
|  |  |  |
| Sunset | '000 sqft | Proportion |
| Commercial | 503 | $5 \%$ |
| Residential | 8,770 | $95 \%$ |
| Total | 9,273 | $100 \%$ |

TABLE 4 - FUTURE PLANNED DWELLING UNITS AND COMMERCIAL INDUSTRIAL SQUARE FEET.

| Projected Growth | Current Planned Land Use |
| :--- | ---: |
| Dwelling Units | 1,603 |
| Commercial + Industrial sq ft | $15,369,129$ |

This potential growth in development was also compared to the Long-Range Plan Model inputs/outputs.

## Casper Travel Demand Model (TDM)

A new advanced travel forecasting model, the Casper Travel Demand Model (TDM), was developed and validated as part of the Casper Area MPO Long Range Plan Update: Connecting Crossroads (Casper Regional Travel Demand Model 2020, N. Marshall, February 2020). The inputs and outputs of the model
base year (2015) and horizon year (2048) future scenarios were also reviewed and compared with historic growth for this study.

The Casper TDM included the following significant enhancements over the MPO's previous version: Estimation of Walk/Bike Trips, Time of Day modeling (AM, Midday, PM, Night), Dynamic Traffic Assignment (DTA), and additional travel time validation (versus just validating to traffic counts). The model uses the TRANSCAD travel forecasting software as a foundation to store inputs/outputs/display results, but also the SmartDTA (a version of DTALite) to carry out the dynamic traffic assignment portions of the process. DTA provides the ability to analyze traffic congestion bottlenecks as they vary throughout the day and other operational attributes (such as signal timing and coordination).

To review the LRP inputs and results, the Casper TDM was obtained and implemented for this study. The model coverage and traffic analysis zones (TAZs), along with the Bar Nunn subarea are shown in Figure 21. The master network file (both current and future) is shown in Figure 22.


FIGURE 21 -- CASPER TDM TRAFFIC ANALYSIS ZONES


FIGURE 22 -- CASPER TDM TRANSPORTATION NETWORK
Six different scenarios were developed as part of the LRP Update. These were combinations of three different network options (No Build, Multimodal, and Auto-Oriented), and two different land use scenarios (compact focusing on the dense areas of Casper, and Suburban which provided a more distributed growth pattern). Within the Bar Nunn subarea, both land use scenarios had the same land use (however, the suburban scenario had additional development east of I- 25 in TAZs 201 and 163). When examining the difference in outputs, the Compact No Build scenario was chosen for review. The change in households for this scenario is shown in Figure 23, and the change in employment is shown in Figure 24. Note that there is very little change in households assumed in the regional LRP and model forecast within Bar Nunn sub area with the exception of TAZs 161 and 203 which are south of the town boundaries. The same is true for the change in employment; however, the percentage change in employment for TAZs 161 and 203 is somewhat less.

Note that the zoning and land use proposed provides for much more growth within the Town of Bar Nunn than the regional LRP assumes. This often happens on the edges of MPO or model regions. The potential growth of trips from the proposed land and failure analysis was therefore used for assessing the 2025 traffic impacts.

The future traffic projections from the model along with past historic growth and potential traffic generated from the new zoning were also examined to derive the near-term growth in traffic for the 2025 network analysis.


FIGURE 23 -- 2015 VERSUS 2048 CHANGE IN HOUSEHOLDS


FIGURE 24 -- 2015 TO 2048 CHANGE IN EMPLOYMENT

## 7. Future Traffic Projections and Traffic Operations Analysis

A future conditions capacity analysis of the Salt Creek Highway corridor was performed to evaluate the impacts of the proposed concepts, to identify the short-term capacity of the corridor, and to assess what future land use and new development can be supported by the remaining roadway capacity of the corridor with the short and mid-term improvements in place.

## 2025 No Build

A future conditions capacity analysis of the Salt Creek Highway corridor was performed using Synchro 10, a macroscopic and deterministic traffic analysis software which implements the Highway Capacity Manual (HCM) methodology. Future 2025 No Build volumes, shown in Figure 25 , were developed by applying a simple annual growth rate to the existing conditions volumes. A $0.75 \%$ annual growth rate was used based on a detailed review of the Casper Regional Travel Demand Model, historical traffic counts, and Annual Population Estimates from the US Census Bureau.

Synchro was used to report the average vehicle delays and Level of Service (LOS) at the six study intersections. The analysis results are presented in Table 5. Detailed Synchro reports are available in Appendix C. The 2025 No-Build analysis shows that all intersections are expected to operate acceptably in 2025. All movements are below capacity and operate at an LOS C or better.

TABLE 5 -- SALT CREEK HIGHWAY CAPACITY ANALYSIS - 2025 NO BUILD

| \# | Intersection | Approach | Existing Conditions AM (PM) |  |  |  | No Build 2025 AM (PM) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{aligned} & \text { Delay/Veh } \\ & \text { (sec) } \end{aligned}$ |  | Level of Service |  | $\begin{aligned} & \text { Delay/Veh } \\ & (\mathrm{sec}) \end{aligned}$ |  | Level of Service |  |
| 1 | Salt Creek Hwy \& Westwinds Rd | Westbound | 3 | (1) | A | (A) | 3 | (1) | A | (A) |
|  |  | Northbound | 9 | (9) | A | (A) | 9 | (9) | A | (A) |
| 2 | Salt Creek Hwy \& McMurry Rd | Eastbound | 9 | (9) | A | (A) | 9 | (9) | A | (A) |
|  |  | Northbound | 7 | (7) | A | (A) | 7 | (7) | A | (A) |
| 3 | Salt Creek Hwy \& Prairie Ln | Eastbound | 9 | (9) | A | (A) | 10 | (9) | A | (A) |
|  |  | Northbound | 2 | (1) | A | (A) | 2 | (1) | A | (A) |
| 4 | Salt Creek Hwy \& Sunset Blvd | Eastbound | 10 | (9) | A | (A) | 10 | (9) | A | (A) |
|  |  | Northbound | 1 | (1) | A | (A) | 1 | (1) | A | (A) |
| 5 | Salt Creek Hwy \& Antelope Dr | Eastbound | 11 | (10) | B | (A) | 11 | (10) | B | (A) |
|  |  | Northbound | 4 | (4) | A | (A) | 4 | (4) | A | (A) |
| 6 | Salt Creek Hwy \& Howard St | Eastbound | 10 | (10) | B | (B) | 11 | (10) | B | (B) |
|  |  | Westbound | 11 | (12) | B | (B) | 12 | (12) | B | (B) |
|  |  | Northbound | 12 | (15) | B | (B) | 13 | (16) | B | (C) |
|  |  | Southbound | 13 | (12) | B | (B) | 14 | (12) | B | (B) |

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FIGURE 25 -- INTERSECTION TRAFFIC VOLUMES - 2025 NO BUILD

## 2025 Build

The concepts which impact traffic operations along Salt Creek Highway were analyzed in Synchro software under 2025 conditions. The concepts included in the analysis are listed below:

- Concept 0a: Full Closure of Antelope Dr.
- Note: Concept 0a was included in the analysis of all other concepts.
- Concept 2: Traffic Control Upgrades (Roundabout at Sunset Blvd.)
- Concept 1 and 6:
- Concept 1: New Left-Turn Lanes along Salt Creek Highway
- Concept 6: Salt Creek Cross-Section Improvements

Concepts 1 and 6 were analyzed using the same Synchro model as they are operationally identical: the two concepts result in the same intersection lane configuration along the corridor.

## Concept 0a

Concept Oa consists of the closure of Antelope Drive at Salt Creek Highway. The 2025 No-Build traffic volumes were rerouted through the study network to account for the closure, as shown in Figure 26. Synchro was used to report the delay and LOS at the six study intersections. The analysis results are presented in Table 6. The Concept 0a analysis shows that all movements at the study intersections are expected remain below capacity and operate at an LOS C or better.

TABLE 6 -- SALT CREEK HIGHWAY CAPACITY ANALYSIS - CONCEPT OA

| \# | Intersection | Approach* | No Build 2025 AM (PM) |  |  |  | Concept 0a 2025 (Antelope Dr. Closed) AM (PM) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Delay/Veh (sec) |  | Level of Service |  | Delay/Veh (sec) |  | Level of Service |  |
| 1 | Salt Creek Hwy \& Westwinds Rd | Westbound | 3 | (1) | A | (A) | 3 | (1) | A | (A) |
|  |  | Northbound | 9 | (9) | A | (A) | 9 | (9) | A | (A) |
| 2 | Salt Creek Hwy \& McMurry Rd | Eastbound | 9 | (9) | A | (A) | 9 | (9) | A | (A) |
|  |  | Northbound | 7 | (7) | A | (A) | 7 | (7) | A | (A) |
| 3 | Salt Creek Hwy \& Prairie Ln | Eastbound | 10 | (9) | A | (A) | 10 | (9) | A | (A) |
|  |  | Northbound | 2 | (1) | A | (A) | 2 | (1) | A | (A) |
| 4 | Salt Creek Hwy \& Sunset Blvd | Eastbound | 10 | (9) | A | (A) | 11 | (10) | B | (A) |
|  |  | Northbound | 1 | (1) | A | (A) | 4 | (4) | A | (A) |
| 5 | Salt Creek Hwy \& Antelope Dr | Eastbound | 11 | (10) | B | (A) | - | (-) |  | $(-)$ |
|  |  | Northbound | 4 | (4) | A | (A) | - | (-) | - | $(-)$ |
| 6 | Salt Creek Hwy \& Howard St | Eastbound | 11 | (10) | B | (B) | 11 | (10) | B | (B) |
|  |  | Westbound | 12 | (12) | B | (B) | 12 | (12) | B | (B) |
|  |  | Northbound | 13 | (16) | B | (C) | 13 | (16) | B | (C) |
|  |  | Southbound | 14 | (12) | B | (B) | 14 | (12) | B | (B) |

*Note: Approaches with free movements and no vehicle delay are omitted


FIGURE 26 -- INTERSECTION TRAFFIC VOLUMES - 2025 ANTELOPE DR CLOSURE

## Concept 2

Concept 2 consists of the conversions of the Sunset Blvd. at Salt Creek Highway intersection to a roundabout. The concept also assumes the closure of Antelope Dr. The Synchro reported intersection delay and LOS are presented in Table 7. Detailed Synchro reports are available in Appendix C. The analysis shows that all movements at the study intersections are expected to remain below capacity and operate at an LOS C or better. The conversion of the Sunset Blvd intersection to a roundabout is expected to reduce side street delays and a slightly increase mainline delays along the Salt Creek Highway approaches.

## Concepts 1 and 6

Concept 1 introduces mainline left-turn lanes at the six study intersections. Concept 6 adds a center turn lane along Salt Creek Highway between Westwinds Road to the north and Howard Street to the south. The two concepts were analyzed using the same Synchro model as they are operationally identical at the study intersections. The Synchro reported intersection delay and LOS are presented in Table 7. The analysis shows that all movements at the study intersections are expected to remain below capacity and operate at an LOS C or better.

TABLE 7 -- SALT CREEK HIGHWAY CAPACITY ANALYSIS - 2025 CONCEPTS 1, 2, AND 6

| \# | Intersection | Approach* | Sunset Roundabout 2025 AM (PM) |  |  |  | Left Turn Lanes 2025-AM <br> (PM) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Delay/Veh (sec) |  | Level of Service |  | Delay/Veh (sec) |  | Level of Service |  |
| 1 | Salt Creek Hwy \& Westwinds Rd | Westbound | 3 | (1) | A | (A) | 3 | (1) | A | (A) |
|  |  | Northbound | 9 | (9) | A | (A) | 9 | (9) | A | (A) |
| 2 | Salt Creek Hwy \& McMurry Rd | Eastbound | 9 | (9) | A | (A) | 9 | (9) | A | (A) |
|  |  | Northbound | 7 | (7) | A | (A) | 7 | (7) | A | (A) |
| 3 | Salt Creek Hwy \& Prairie Ln | Eastbound | 10 | (9) | A | (A) | 10 | (9) | A | (A) |
|  |  | Northbound | 2 | (1) | A | (A) | 2 | (1) | A | (A) |
| 4 | Salt Creek Hwy \& Sunset Blvd | Eastbound | 5 | (4) | A | (A) | 11 | (10) | B | (A) |
|  |  | Northbound | 4 | (5) | A | (A) | 4 | (4) | A | (A) |
|  |  | Southbound** | 5 | (4) | A | (A) | - | $(-)$ | - | $(-)$ |
| 5 | Salt Creek Hwy \& Antelope Dr | Eastbound | - | (-) | - | $(-)$ | - | (-) | - | $(-)$ |
|  |  | Northbound | - | (-) | - | (-) | - | (-) | - | (-) |
| 6 | Salt Creek Hwy \& Howard St | Eastbound | 11 | (10) | B | (B) | 10 | (10) | B | (B) |
|  |  | Westbound | 12 | (12) | B | (B) | 12 | (12) | B | (B) |
|  |  | Northbound | 13 | (16) | B | (C) | 12 | (15) | B | (B) |
|  |  | Southbound | 14 | (12) | B | (B) | 14 | (12) | B | (B) |

*Note: Approaches with free movements and no vehicle delay are omitted
**The southbound approach of Salt Creek Hwy \& Sunset Blvd only experiences vehicle delays under the Sunset Roundabout 2025 alternative.

## Corridor Spare Capacity

The spare capacity of the Salt Creek Highway corridor with all short and mid-term build improvements was evaluated using the Synchro model. Volumes were increased uniformly at each intersection until one of the movements stopped operating acceptably (LOS D or worse). The capacity evaluation assumed that Concept 0a (closure of Antelope Dr) and Concept 2 (Roundabout at Sunset Blvd) were implemented.

The intersection of Sunset Blvd and Salt Creek Highway has the least spare capacity, excluding the Howard St intersection as it is located outside the Bar Nunn town limits. The resulting failure volumes are presented in Figure 27. It is estimated that the corridor could accommodate several hundred additional peak hour vehicle trips at each intersection approach prior to failing.

## Land Use / Development Thresholds

As the Sunset Blvd and Salt Creek Highway intersection has the least spare capacity, it was used to assess what additional land use can be supported within the corridor. The spare capacity was calculated as the difference of the failure traffic volumes and the 2025 Build traffic volumes. This spare capacity was then assigned to potential land uses according to the Institute of Transportation Engineer's Trip Generation Manual, $10^{\text {th }}$ Edition trip rates. The spare capacity was assigned to an even mix of residential, commercial, and industrial land use using ITE land use codes and trip rates shown in Table 8. With the short-term and mid-term improvements in place, the corridor can accommodate approximately an additional 621 dwelling units, 587,000 gross square feet of commercial land use, and 335,000 gross square feet of industrial land use, as shown in Table 9.

TABLE 8 -- ITE TRIP RATES

| Zoning <br> (Land Use) | Land Use | Trip Rates |  | Trip Rate |
| :--- | :---: | :---: | :---: | :---: |
|  |  | AM | PM | Unit |
| Residential <br> (Single-Family Detached Housing) | 210 | 0.74 | 0.99 | Dwelling Unit |
| Commercial <br> (Industrial Park) | 130 | 0.40 | 0.40 | 1000 Sq. Ft. GFA |
| Industrial <br> (General Light Industrial) | 110 | 0.70 | 0.63 | 1000 Sq. Ft. GFA |

TABLE 9 -- ADDITIONAL LAND USE ACCOMMODATED BY SPARE CORRIDOR CAPACITY

| Peak <br> Period | Corridor <br> Trip <br> Capacity | Dwelling Units | Commercial |  | Industrial |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Jobs | Area <br> [sqft] | Jobs |  |
| AM | 939 | 634 | 586,644 | 587 | 335,225 | 436 |
| PM | 1,229 | 621 | 768,300 | 768 | 487,810 | 634 |
| Add. Land Use | - | 621 | $\mathbf{5 8 6 , 6 4 4}$ | 587 | $\mathbf{3 3 5 , 2 2 5}$ | $\mathbf{4 3 6}$ |



FIGURE 27-INTERSECTION TRAFFIC VOLUMES - FAILURE VOLUMES

## Mitigation Strategies to Accommodate Additional Land Use/Development Capacity

The current planned land use for the ultimate build out of Bar Nunn is greater than what can be accommodated by the corridor with the short and mid-term roadway improvements. A comparison of the current planned land use, the land use growth from the Casper TDM model, and the land use that can be accommodated by the Salt Creek Highway corridor (failure year analysis) is shown in Figure 28.

The capacity along the study corridor can be increased to accommodate additional future developments through several further traffic control, traffic management and intersection improvement strategies to extend the life of the short and mid-term improvements:

- Widening Salt Creek Highway from one to two through lanes per direction
- Introducing separate right and left-turn lanes at cross streets
- Installing channelized right-turns with acceleration lanes for cross streets
- Diversions of trips to the Westwinds Rd interchange that are now using the southern portions of Salt Creek Highway to enter the Town
- Installing traffic signals at key intersections along the corridor

Changing the intersection operation acceptability threshold from LOS C to LOS D or E would also permit the corridor to accommodate more new trips and related development sites.

Approximate Land Use Growth Forecasts by Source


FIGURE 28 - COMPARISON OF LAND USE FORECASTS

## 8. Initial Construction Cost Estimates

Construction cost estimates were developed for the retained alternatives using a major quantities cost estimating methodology. Major quantities estimates are used to estimate construction costs during the planning stage and early in the preliminary engineering stage. The idea is to estimate as accurately as possible those categories that can be estimated in the very early stages such as grading, paving, structures and shoulders items and calculate the remaining categories as percentages of those categories. A total of nine separate engineering categories were used for estimates including mobilization, earthwork, drainage, structures, paving, shoulders, traffic, landscaping and utilities.

Detailed cost estimate worksheets can be found in Appendix D . The quantities used in each cost estimate were based on the conceptual engineering plans. The total cost and unit prices are calculated in present value dollars. Adjustments should be made for inflating costs to future years if improvements are delayed. Contingency percentages are consistent with WYDOT planning-level cost estimate practices. The raw construction costs do not include engineering design fees, right-of-way acquisition or utility relocation costs but they may be covered under contingent percentage. Lastly, the costs assume full resurfacing of the roadway (e.g., mill and overlay) for any partial roadway work.

The cost estimates include a no-build cost, which is the cost of maintaining and rehabilitating Antelope Drive. The build costs assume that Antelope Drive is closed and removed and that the future maintenance funds are repurposed for constructing one or more of the short or mid-term alternatives. If Antelope Drive remains open, and one or more short or mid-term alternatives are built, the total cost estimate would need to include the cost of the No Build (e.g. Antelope Drive rehabilitation) as well.

Table 10 summarizes the cost estimates for each alternative.
TABLE 10 -- SUMMARY OF COST ESTIMATES FOR RETAINED ALTERNATIVES

| Alternative | Cost |
| :--- | :--- |
| No Build (Antelope Drive Rehabilitation) | $\$ 500,000-\$ 600,000$ |
| Short-Term Concept 1 Left Turn Lanes | $\$ 400,000-\$ 500,000(\$ 100,000$ per int.) |
| Short-Term Concept 2 Roundabout | $\$ 500,000-\$ 600,000$ |
| Short-Term Concept 3 Channelization | $\$ 500,000-\$ 600,000$ |
| Short-Term Concept 4 Streetscape | $\$ 900,000-\$ 1,000,000$ |
| Short-Term Concept 5 Truck Access Road | $\$ 600,000-\$ 700,000$ |
| Mid-Term Concept 6 Salt Creek 3 Lane Section + Path | $\$ 1,900,000-\$ 2,000,000$ |
| Mid-Term Concept 7 Salt Creek Side Path | $\$ 200,000-\$ 300,000$ |

## Potential Impacts

Right-Of-Way - It is anticipated that the roundabout, left-turn lanes, truck access road and shared use path may require additional right-of-way or easements. Potentially land swaps may be possible at the intersection with Sunset Blvd between WYDOT and the Town.

Utilities - The existing open drainage swale and associated culverts on the west side of Salt Creek Highway would be impacted with the construction of the roundabout, left-turn lanes or shared use path. Under the widening of Salt Creek, it is anticipate that a closed drainage system would be installed.

Environmental - The improvements to Salt Creek Highway for individual left-turn lanes or mid-term corridor widening would impact the existing salt flats and would require underdrains and subsurface stabilization as part of any roadwork. It is suggested that funds for the long-term rehabilitation of Salt Creek be provided to the Town as part of the roadway transfer agreement with WYDOT.

## 9. Measures of Effectiveness and Screening

The retained alternatives were evaluated based on specific measures of effectiveness (MOE) selected in consultation with the MPO and Town. The screening of the MOEs was performed in two levels.

## Level 1 Screening

The first level of screening evaluated 15 initial alternatives, ideas, and concepts to determine whether they should advance to a more detailed screening. This screening step first assessed whether the outcomes associated with the concept would meet the goals and objectives the Town has laid out for this project. Ten of the initial concepts met this criterion, while the remaining five did not, for the reasons outlined below.

- Partial Closure of Antelope Drive: this would not effectively address the maintenance challenges of Antelope Drive
- I-25 Guide Signing: by itself, this would not effectively address the desire for a gateway treatment to the Town, nor would it have a meaningful impact on traffic patterns along Salt Creek Highway
- All-way stop or signal control at the Salt Creek Highway/Sunset Blvd intersection: these treatments are not warranted based on traffic volumes, nor would they serve as an inviting gateway treatment to the Town.
- Roundabout at the Salt Creek Highway and Antelope Drive intersection: maintaining even some traffic on Antelope Drive between Salt Creek Highway and the former Arena Street intersection would not meet the Town's goals; therefore, there is no reason to construct a roundabout at the intersection.

A second criterion, whether a concept was feasible within the short- to mid-term timeframe of this study, eliminated one additional concept: extending Coyote Avenue to Wardwell Industrial Avenue. Between these two criteria, nine concepts advanced to a Level 2 evaluation described below. Table 11, below, summarizes the Level 1 screening.

TABLE 11 -- LEVEL 1 SCREENING MATRIX

| Alternative | Type | Description | Meets Community and Project Goals | Feasibility |
| :---: | :---: | :---: | :---: | :---: |
| Alt 0A | Street <br> Pattern | Full Closure of Antelope Drive <br> Remove existing pavement between Salt Creek and Arena and divert traffic to Sunset Blvd | Yes | Yes |
| Alt 0B | Street <br> Pattern | Partial Closure of Antelope Drive <br> Restrict westbound traffic between Salt Creek and Arena and divert traffic to Sunset Blvd | No | No |
| Alt 0C | Wayfinding | I-25 Guide Signing <br> Modify/Replace I-25 exit signs to sign Westwinds as the Bar Nunn interchange exit | No | Yes |
| Alt 0D | Wayfinding | Gateway Marquee <br> Install large monument Bar Nunn welcome sign at Salt Creek/Sunset | Yes | Yes |
| Alt 1 | Safetyl <br> Capacity | Salt Creek Highway Left Turn <br> Lanes <br> Add northbound left turn lanes on Salt Creek Highway at Sunset, Prairie, New Truck Road, Coyote, and McMurry. Include storage for stopping distance and appropriate tapers. | Yes | Yes |
| Alt 2-A | Traffic <br> Control | Roundabout at Salt Creek Highway <br> \& Sunset Blvd <br> Construct a modern roundabout at the intersection, to accommodate trucks, buses and emergency vehicles. | Yes | Yes |
| Alt 2-B | Traffic <br> Control | All-Way Stop Control at Salt Creek Highway \& Sunset Blvd Install all-way stop signs on each approach. | No | Yes |
| Alt 2-C | Traffic <br> Control | Signal Control at Salt Creek Highway \& Sunset Blvd Install a new traffic signal | No | Yes |


| Alternative | Type | Description | Meets Community and Project Goals | Feasibility |
| :---: | :---: | :---: | :---: | :---: |
| Alt 2-D | Traffic <br> Control | Roundabout at Salt Creek Highway <br> \& Antelope Drive <br> Construct a modern roundabout at the intersection, to accommodate trucks, buses and emergency vehicles. | No | No |
| Alt 3 | Traffic <br> Calming | Antelope/Sunset Intersection Improvements <br> Install curb extensions, crosswalks, all-way stop signs and stop bars. | Yes | Yes |
| Alt 4 | Streetscape | Streetscape Improvements on Sunset Blvd <br> Wide decorative sidewalks, plantings, pedestrian lighting, street trees, pocket park between Salt Creek Highway and Antelope Drive. | Yes | Yes |
| Alt 5A | Truck Route | New Truck Route to Wardwell Industrial Avenue <br> Construct a new road between Prairie and Coyote. | Yes | Yes |
| Alt 5B | Truck Route | New Truck Route to Wardwell Industrial Avenue <br> Extend Coyote Avenue to Wardell Industrial Avenue | Yes | No |
| Alt 6 | Safety/ Capacity | Mid-Term Widening of Salt Creek Highway <br> Widen to two lanes per direction between Howard Street and the widened section north of McMurry, with left turn lanes at intersections and enclosed drainage/curbs. | Yes | Yes |
| Alt 7 | Multimodal | Shared Use Path on the West Side of Salt Creek Highway <br> Construct multi-use path from Howard to Antelope Drive. | Yes | Yes |

## Level 2 Evaluation

The second level of screening evaluated the nine concepts that advanced from the Level 1 screening for their benefits and impacts across the following categories:

- Construction costs - Traffic operations benefits
- Maintenance costs
- Multi-modal benefits
- Aesthetic value
- Truck traffic reduction
- Traffic safety benefit
- Community/ Town support


## Construction costs

The lowest-cost concepts are the full closure of Antelope Drive (Concept \#OA) and the installation of the marquee sign (Concept \#OD), while the highest-cost concepts are the road construction projects that would install streetscaping elements (Concept \#4), create a new truck route to Wardwell Industrial Avenue (Concept \#5A) and widen Salt Creek Highway (Concept \#6).

## Maintenance costs

Fully closing Antelope Drive between Salt Creek Highway and the former Arena Street intersection (Concept \#OA) would result in a reduction in maintenance costs. Among the other improvement concepts, the gateway marquee sign (Concept \#OD), Salt Creek Highway/Sunset Boulevard roundabout (Concept \#2C), and Antelope/Sunset intersection improvements (Concept \#3) would have the lowest ongoing maintenance costs, while the mid-term widening of Salt Creek Highway (Concept \#6) would have the highest ongoing maintenance costs.

## Aesthetic value

The gateway marquee sign (Concept \#OD), roundabout and streetscape improvements (Concepts \#2C through \#4) and shared-use path (Concept \#7) would add aesthetic value to the Town, while the other concepts would be aesthetically neutral.

## Traffic safety benefits

The Salt Creek Highway and Sunset Boulevard roundabout (Concept \#2C) and Antelope/Sunset intersection improvements (Concept \#3) would likely have the greatest traffic safety benefit, The Antelope Drive closure (Concept \#0A), new left turn lanes (Concept \#1), new truck route (Concept \#5), Salt Creek Highway widening (Concept \#6), and multi-use pathway (Concept \#7) would have lesser benefits. None of the concepts would be detrimental to traffic safety.

## Traffic operational benefits

The new truck route to Wardwell Industrial Avenue (Concept \#5) and widening of Salt Creek Highway (Concept \#6) would have more operational benefits than the Antelope Drive closure (Concept \#0A), leftturn lanes along Salt Creek Highway (Concept \#1), or roundabout at Salt Creek Highway and Sunset Boulevard (Concept \#2C). None of the concepts would likely be detrimental to traffic operations.

## Multi-modal benefits

Fully closing Antelope Drive (Concept \#OA), constructing a roundabout at Salt Creek Highway and Sunset Boulevard (Concept \#2C), improving the intersection at Salt Creek Highway and Antelope Drive (Concept \#3), adding streetscape improvements along Sunset Boulevard (Concept \#4), and creating a shared-use pathway along Salt Creek Highway (Concept \#7) would all improve multimodal accessibility within the Town.

## Reduced Truck Traffic on Salt Creek Highway

Current truck traffic volumes range from approximately $3 \%$ to $12 \%$ during peak hours, according to traffic counts collected as part of this study. The continued use of Salt Creek Highway by trucks will result in deterioration of the roadway and additional maintenance costs by the Town, particularly in the area between Sunset Blvd and Antelope Drive where the salt flats lie. Extending Salt Creek Highway's pavement life cycle by reducing truck traffic along Salt Creek can be accomplished by installing new traffic controls and encouraging trucks to utilize the Westwinds interchanges to access Westwinds Blvd and points north of Sunset Blvd. Concept \#2C (roundabout at Sunset Blvd) would best meet this objective.

## Community/Town support

Fully closing Antelope Drive between Salt Creek Highway and the former Arena Street intersection (Concept \#OA) may have moderate quality-of-life impacts and face challenges in acceptance from Town residents. However, the public should expect few to no quality-of-life drawbacks resulting from any of the other concepts.

Table 12, below, summarizes the Level 2 screening.

TABLE 12 -- LEVEL 2 EVALUATION MATRIX

| Concept |  |  |  |  | Screening Criteria |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Type |  | Desc |  | $\begin{aligned} & \text { ㅇ } \\ & \text { 응 } \\ & \text { en } \\ & \text { 릉 } \\ & \text { B } \\ & 0 \\ & 0 \end{aligned}$ | $\begin{aligned} & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \end{aligned}$ |  |  |  |  |  |  |  |
| Alt | Street Pattern | Full Closure of Antelope Drive - Remove existing pavement between Salt Creek and Arena and divert traffc to Sunset Blvd |  |  |  |  |  |  |  |  |  |  |  |
| Alt | Wayfindi ng | Gateway Marquee - Install large monument Bar Nunn welcome sign at Salt Creek/ Sunset |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{gathered} \text { Alt } \\ 1 \end{gathered}$ | Safety/ Capacity | Salt Creek Highway Left Turn Lanes - Add northbound left turn lanes on Salt Creek Highway at Sunset, Prairie, New Truck Road, Coyote, and McMurry. Include storage for stopping distance and appropriate tapers. |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \text { Alt } \\ & \text { 2C } \end{aligned}$ | Traffic Control | Roundabout at Salt Creek Highway \& Sunset Blvd - Construct a modern roundabout at the intersection, to accommodate trucks, buses and emergency vehicles. |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{gathered} \text { Alt } \\ 3 \end{gathered}$ | Traffic Calming | Antelope/Sunset Intersection Improvements - Install curb extensions, crosswalks, all-way stop signs and stop bars. |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{gathered} \text { Alt } \\ 4 \end{gathered}$ | Streetsca pe | Streetscape Improvements on Sunset Blvd - Wide decorative sidewalks, plantings, pedestrian lighting, street trees, pocket park between Salt Creek Highway and Antelope Drive. |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \text { Alt } \\ & 5 A \end{aligned}$ | Truck Route | New Truck Route to Wardwell Industrial Avenue - Construct a new road between Prairie and Coyote. |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{gathered} \text { Alt } \\ 6 \end{gathered}$ | Safety/ Capacity | Mid-Term Widening of Salt Creek Highway - Widen to two lanes per direction between Howard Street and the widened section north of McMurry, with left turn lanes at intersections and enclosed drainage/curbs. |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{gathered} \text { Alt } \\ 7 \end{gathered}$ | Multimod al | Shared Use Path on the West Side of Salt Creek Highway - construct multi-use path from Howard to Antelope Drive. |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| High Impact/ Low Benefit |  |  | Medium Impact/ Medium Benefit | Low Impact/ Slight Benefit | Neutral Impact/ <br> Neutral Benefit |  |  |  |  | $\begin{gathered} \text { No Ir } \\ \text { High } \end{gathered}$ | mpact <br> Benef |  |  |

## 10. Second Public Meeting

To solicit community and stakeholder input on the potential alternative solutions, a second public open house was held on January 14, 2020 at The Hangar from 4 PM to 7 PM. The meeting was simulcast through the MPO via a Microsoft Teams meeting that was accessible via the MPO's social media page. Meeting announcements were posted on the MPO's social media account and in social media Town newsletter. The meeting included a short presentation on the study purpose and goals and objectives, plus existing data and conditions, followed by an open house with roll plans and posters of the proposed alternative solutions. Approximately 15 people attended in person and about 5 attended virtually. The attendees were asked to provide feedback on proposed access improvements to the Town.

Favorable comments were received by community members for the left-turn lanes, streetscape, truck route and roundabout. Unfavorable comments were received on the roundabout. Written public comments are included in Appendix E.


## 11. Recommendations and Implementation

After review of the Phase 2 screening summarized in Table 12, and considering town citizen, council and mayor input, a series of short- and mid-term solutions were recommended. Public/community acceptance ultimately determines the true viability of any solutions. Cost and the availability of funds are critical factors that must be at the forefront of determining recommended solutions.

Arguably, the most effective mid-term improvement on Salt Creek Highway is also the most expensive. Widening Salt Creek Highway to 3-lane section would cost approximately $\$ 2,000,000$. The cost for a roundabout is approximately $\$ 600,000$, while each left turn lane would cost approximately $\$ 100,000$. The streetscape cost is approximately $\$ 1,000,000$ and the multimodal option (shared use path alone) cost is \$300,000.

The following alternatives are recommended, in the following priority order:
a. Short-Term: Remove or discontinue the maintenance of the pavement on the section of Antelope Drive near Salt Creek Highway. If removed, seek an opportunity to include a multi-use path connection in the former Antelope Drive right-of-way. If remained open, consider implementing a peak hour 'No Left-Turn' prohibition once other Salt Creek Highway intersections are improved.
b. Short-Term: Construct a new roundabout at Sunset Blvd. and extend the taper/ transition to provide a leftturn lane at Prairie Lane. The roundabout will serve as a traffic calming treatment, enhance traffic safety, encourage diversion of traffic entering and exiting the Town to other streets and interchanges, and reduce truck traffic, prolonging the life of Salt Creek Highway.
c. Short-Term: Construct the channelization improvements along Sunset Blvd and Prairie Lane at Antelope. The design should follow the initial tactical treatments already initiated by the Town to potentially reduce construction costs while maintaining the desired traffic calming and streetscape elements.
d. Mid-Term: Construct the streetscape improvements along Sunset Blvd between Salt Creek and Antelope Drive. This will provide the opportunity to build an attractive and more visible gateway to Bar Nunn, plus a "town center" area. It is recommended that this project occur as part of the new Town Hall construction or other redevelopment so that the site plan and streetscape design can be best coordinated, and the public realm most efficiently activated.
"When the project began, we knew that we needed to address a multitude of concerns and issues, but we were not sure what exactly would be the best solution. This is exactly why we wanted to engage in this study. Living in Wyoming my entire life, I have not spent much time using roundabouts and as such was not particularly open to the idea of a roundabout as any part of the solution to our issues. However, after talking with consultants and engineers, along with reviewing the data and statistics of the various options presented, it became clear that the roundabout suggested in this study is the best option for solving our issues. It is clearly the most costeffective solution that provides for continual and safer traffic flows."

Mayor Patrick Ford, Town of Bar Nunn
e. Mid-Term: Construct the new road from Salt Creek Highway to the Wardwell Industrial Avenue. Construct a left turn lane on Salt Creek Highway
f. Long-Term: Construct the shared use path along the west side of Salt Creek Highway from Sunset Blvd. to Howard Street. Provide room for possible future widening of Salt Creek Highway.

In addition to the above capital improvements, the following non-capital solutions should be investigated further and considered:

1) Coordinate with the school district to relocate and/or improve bus stops. Provide pull-out areas where possible.
2) Increase enforcement of speeds, truck routes and traffic control compliance.
3) Coordinate with property owners to secure needed right-of-way, including the KOA and WYDOT.

Funding is always a challenge to secure. With the change of federal administration, there is an increased possibility of substantial federal transportation infrastructure investment. To secure these funds, Wyoming is likely to need to provide additional matching funds for the state match. There are various grant programs available including BUILD, Transportation Alternatives Program, Safe Routes to School, Highway Safety Improvement Program, Energy/ Mineral Grant. Additionally, new programs may be added in the future. Opportunities for public/private partnerships with developers should be pursued, whenever possible.

Finally, is recommended that the Casper Area MPO and the Town of Bar Nunn advance the study recommendations into preliminary and final design and list the projects in the local Transportation Improvement Program (TIP) so that Bar Nunn is in an advantageous position to compete for and secure funding.

## Appendix A. Crash Data

TABLE 13. CRASHES AT SALT CREEK HIGHWAY/HOWARD STREET, 2004-14; 2018-19

| Crash Type and Direction | Number of Crashes |
| :---: | :---: |
| Unknown |  |
| Angle Direction not Specified | 2 |
| Other | 4 |
| Angle (force exceeds 15 degrees) |  |
| Angle (Front to Side), Opposing Direction | 6 |
| Angle Right (Front to Side, includes Broadside) | 8 |
| Angle Same Direction (Front to Side) | 1 |
| Opposing (Opposite Direction within 15 degrees |  |
| Angle (Front to Side), Opposing Direction | 2 |
| Head On (Front to Front) | 1 |
| Rear to Front (Normally Backing) | 1 |
| Passing (glancing collision from same direction) |  |
| Sideswipe Same Direction (Passing) | 2 |
| Same (same direction within 15 degrees) |  |
| Rear End (Front to Rear) | 4 |
| Grand Total | 31 |
| Crash Severity | Number of Crashes |
| Incapacitating Injury | 1 |
| No Injury | 20 |
| Non-Incapacitating Injury | 2 |
| Possible Injury | 4 |
| Property Damage Only | 4 |
| Grand Total | 31 |

TABLE 13 CONTINUED

| Weather and Roadway Conditions |  |
| :--- | ---: |
| Clear |  |
| Dry |  |
| Darkness Unlighted | 3 |
| Clear |  |
| Roadway Condition Unknown |  |
| Dry | 1 |
| Ice/Frost | 2 |
| Snow | 1 |
| Dawn | 1 |
| Clear |  |
| Dry | 1 |
| Daylight | 1 |
| Clear | 1 |
| Roadway Condition Unknown | 1 |
| Dry | 1 |
| Snow | 1 |
| Wet | 1 |
| Cloudy, Overcast | 1 |
| Unknown | 1 |
| Raining | 1 |
| Roadway Condition Unknown | 1 |
| Water Standing/Running | 1 |
| Wet | 1 |
| Snowing | 1 |
| Roadway Condition Unknown | 1 |
| Ice/Frost | 1 |
| Grand Total | 1 |

TABLE 14. CRASHES AT SALT CREEK HIGHWAY/ANTELOPE DRIVE, 2004-14; 2018-19

| Crash Type and Direction |  |
| :--- | ---: |
| Unknown |  |
| Other |  |
| Angle (force exceeds 15 degrees) |  |
| Angle (Front to Side), Opposing Direction |  |
| Angle Same Direction (Front to Side) |  |
| Meeting (glancing collision from opposite direction) | 1 |
| Angle (Front to Side), Opposing Direction |  |
| Opposing (Opposite Direction within 15 degrees |  |
| Head On (Front to Front) |  |
| Same (same direction within 15 degrees) |  |
| Rear End (Front to Rear) | 1 |
| Grand Total | 1 |


| Crash Severity | Number of Crashes |
| :--- | ---: |
| Incapacitating Injury | 1 |
| No Injury | 8 |
| Possible Injury | 1 |
| Grand Total | 10 |


| Weather and Roadway Conditions | Number of Crashes |
| :---: | :---: |
| Darkness Lighted |  |
| Snowing |  |
| Ice/Frost | 2 |
| Snow | 1 |
| Daylight |  |
| Clear |  |
| Roadway Condition Unknown | 1 |
| Dry |  |
| Ice/Frost | 1 |
| Grand Total | 10 |

TABLE 15. CRASHES ELSEWHERE IN THE STUDY AREA (MP 16.26 TO MP 18.82); 2004-14; 2018-19

| Crash Type and Direction | Number of Crashes |
| :---: | :---: |
| Unknown |  |
| Other | 1 |
| Unknown | 3 |
| Angle (force exceeds 15 degrees) |  |
| Angle (Front to Side), Opposing Direction | 2 |
| Angle Right (Front to Side, includes Broadside) | 3 |
| Rear to Side (Normally Backing) | 2 |
| Meeting (glancing collision from opposite direction) |  |
| Sideswipe Opposite Direction (Meeting) | 1 |
| Passing (glancing collision from same direction) |  |
| Not a Collision w/2 Vehicles in Transport | 1 |
| Same (same direction within 15 degrees) |  |
| Not a Collision w/2 Vehicles in Transport | 9 |
| Rear End (Front to Rear) | 7 |
| Unknown |  |
| Not a Collision w/2 Vehicles in Transport | 1 |
| Grand Total 30 |  |
| Crash Severity | Number of Crashes |
| Fatal | 1 |
| Incapacitating Injury | 2 |
| No Injury | 15 |
| Non-Incapacitating Injury | 3 |
| Possible Injury | 4 |
| Property Damage Only | 5 |
| Grand Total | 30 |

TABLE 15 CONTINUED

| Weather and Roadway Conditions | Number of Crashes |
| :---: | :---: |
| Darkness Lighted |  |
| Clear |  |
| Dry | 1 |
| Darkness Unlighted |  |
| Blowing Snow |  |
| Ice/Frost | 1 |
| Clear |  |
| Unknown | 3 |
| Dry | 4 |
| Ice/Frost | 2 |
| Snow | 2 |
| Unknown |  |
| Dry | 1 |
| Dawn |  |
| Clear |  |
| Dry | 1 |
| Daylight |  |
| Blowing Snow |  |
| Ice/Frost | 1 |
| Clear |  |
| Unknown | 1 |
| Dry | 8 |
| Ice/Frost | 1 |
| Wet | 1 |
| Snowing |  |
| Ice/Frost | 2 |
| Snow | 1 |
| Grand Total | 30 |

## Appendix B. Concepts




EXISTING SIGNAGE ON I-25
REVISED SIGNAGE ON I-25



Mead§Hunt




赖 $\boldsymbol{C L H}$ Mead
Associates, LLC \&IUN



婁CLH Mead
Associates, LLC \& Ilunt



## Appendix C. Capacity Analysis Worksheets

## Existing Conditions AM

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.3 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 1 |  |  | 4 | i | $\mathbf{7}$ |
| Traffic Vol, veh/h | 114 | 2 | 10 | 18 | 1 | 13 |
| Future Vol, veh/h | 114 | 2 | 10 | 18 | 1 | 13 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 124 | 2 | 11 | 20 | 1 | 14 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 8 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | r |  |  | -1 | 个 |  |
| Traffic Vol, veh/h | 8 | 162 | 48 | 6 | 9 | 3 |
| Future Vol, veh/h | 8 | 162 | 48 | 6 | 9 | 3 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 9 | 176 | 52 | 7 | 10 | 3 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.4 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | M |  |  | -1 | $\uparrow$ |  |
| Traffic Vol, veh/h | 1 | 23 | 23 | 54 | 178 | 0 |
| Future Vol, veh/h | 1 | 23 | 23 | 54 | 178 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 25 | 25 | 59 | 193 | 0 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 4.4 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | $\uparrow$ | $\boldsymbol{T}$ |  |
| Traffic Vol, veh/h | 0 | 171 | 78 | 91 | 231 | 0 |
| Future Vol, veh/h | 0 | 171 | 78 | 91 | 231 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 186 | 85 | 99 | 251 | 0 |



| Intersection |  |
| :--- | ---: | :--- |
| Intersection Delay, s/veh | 12.4 |
| Intersection LOS | B |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | * |  |  | $\uparrow$ | 「 |  | \& |  | ${ }^{7}$ | F |  |
| Traffic Vol, veh/h | 1 | 18 | 8 | 109 | 7 | 180 | 4 | 91 | 107 | 243 | 180 | 1 |
| Future Vol, veh/h | 1 | 18 | 8 | 109 | 7 | 180 | 4 | 91 | 107 | 243 | 180 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 20 | 9 | 118 | 8 | 196 | 4 | 99 | 116 | 264 | 196 | 1 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 1 | 1 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 2 |  |  | 1 |  |  | 2 |  |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 2 |  |  | 1 |  |  | 1 |  |  | 2 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 2 |  |  | 2 |  |  | 1 |  |  |
| HCM Control Delay | 10.3 |  |  | 11.4 |  |  | 12.3 |  |  | 13.3 |  |  |
| HCM LOS | B |  |  | B |  |  | B |  |  | B |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | WBLn2 | SBLn1 | SBLn2 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $2 \%$ | $4 \%$ | $94 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $45 \%$ | $67 \%$ | $6 \%$ | $0 \%$ | $0 \%$ | $99 \%$ |
| Vol Right, \% | $53 \%$ | $30 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 202 | 27 | 116 | 180 | 243 | 181 |
| LT Vol | 4 | 1 | 109 | 0 | 243 | 0 |
| Through Vol | 91 | 18 | 7 | 0 | 0 | 180 |
| RT Vol | 107 | 8 | 0 | 180 | 0 | 1 |
| Lane Flow Rate | 220 | 29 | 126 | 196 | 264 | 197 |
| Geometry Grp | 6 | 6 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.36 | 0.056 | 0.243 | 0.312 | 0.47 | 0.322 |
| Departure Headway (Hd) | 5.91 | 6.833 | 6.925 | 5.738 | 6.411 | 5.901 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 608 | 523 | 518 | 626 | 563 | 610 |
| Service Time | 3.95 | 4.891 | 4.666 | 3.478 | 4.147 | 3.637 |
| HCM Lane V/C Ratio | 0.362 | 0.055 | 0.243 | 0.313 | 0.469 | 0.323 |
| HCM Control Delay | 12.3 | 10.3 | 11.9 | 11.1 | 14.7 | 11.4 |
| HCM Lane LOS | B | B | B | B | B | B |
| HCM 95th-tile Q | 1.6 | 0.2 | 0.9 | 1.3 | 2.5 | 1.4 |

## Existing Conditions PM

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.8 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  | 1 | 个 | a | $\mathbf{7}$ |
| Traffic Vol, veh/h | 24 | 0 | 7 | 52 | 2 | 12 |
| Future Vol, veh/h | 24 | 0 | 7 | 52 | 2 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 26 | 0 | 8 | 57 | 2 | 13 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.3 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | -1 | a |  |
| Traffic Vol, veh/h | 2 | 84 | 125 | 12 | 3 | 4 |
| Future Vol, veh/h | 2 | 84 | 125 | 12 | 3 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 91 | 136 | 13 | 3 | 4 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | r |  |  | $\uparrow$ | 个 |  |
| Traffic Vol, veh/h | 2 | 22 | 29 | 130 | 95 | 5 |
| Future Vol, veh/h | 2 | 22 | 29 | 130 | 95 | 5 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 24 | 32 | 141 | 103 | 5 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.7 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Yr |  |  | -1 | F |  |
| Traffic Vol, veh/h | 0 | 94 | 158 | 192 | 129 | 0 |
| Future Vol, veh/h | 0 | 94 | 158 | 192 | 129 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 102 | 172 | 209 | 140 | 0 |



| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh | 12.8 |
| Intersection LOS | B |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | \$ |  |  | $\uparrow$ | 「 |  | \& |  | ${ }^{*}$ | F |  |
| Traffic Vol, veh/h | 1 | 7 | 4 | 103 | 17 | 234 | 8 | 174 | 105 | 174 | 88 | 1 |
| Future Vol, veh/h | 1 | 7 | 4 | 103 | 17 | 234 | 8 | 174 | 105 | 174 | 88 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 8 | 4 | 112 | 18 | 254 | 9 | 189 | 114 | 189 | 96 | 1 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 1 | 1 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 2 |  |  | 1 |  |  | 2 |  |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 2 |  |  | 1 |  |  | 1 |  |  | 2 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 2 |  |  | 2 |  |  | 1 |  |  |
| HCM Control Delay | 10.1 |  |  | 11.8 |  |  | 14.9 |  |  | 12 |  |  |
| HCM LOS | B |  |  | B |  |  | B |  |  | B |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | WBLn2 | SBLn1 | SBLn2 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $3 \%$ | $8 \%$ | $86 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $61 \%$ | $58 \%$ | $14 \%$ | $0 \%$ | $0 \%$ | $99 \%$ |
| Vol Right, \% | $37 \%$ | $33 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 287 | 12 | 120 | 234 | 174 | 89 |
| LT Vol | 8 | 1 | 103 | 0 | 174 | 0 |
| Through Vol | 174 | 7 | 17 | 0 | 0 | 88 |
| RT Vol | 105 | 4 | 0 | 234 | 0 | 1 |
| Lane Flow Rate | 312 | 13 | 130 | 254 | 189 | 97 |
| Geometry Grp | 6 | 6 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.51 | 0.025 | 0.243 | 0.394 | 0.348 | 0.164 |
| Departure Headway (Hd) | 5.882 | 6.828 | 6.716 | 5.571 | 6.633 | 6.117 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 611 | 523 | 535 | 646 | 543 | 586 |
| Service Time | 3.919 | 4.893 | 4.455 | 3.31 | 4.375 | 3.86 |
| HCM Lane V/C Ratio | 0.511 | 0.025 | 0.243 | 0.393 | 0.348 | 0.166 |
| HCM Control Delay | 14.9 | 10.1 | 11.6 | 11.9 | 12.9 | 10.1 |
| HCM Lane LOS | B | B | B | B | B | B |
| HCM 95th-tile Q | 2.9 | 0.1 | 0.9 | 1.9 | 1.5 | 0.6 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  | 1 | 个 | a | $\mathbf{7}$ |
| Traffic Vol, veh/h | 118 | 2 | 10 | 19 | 1 | 13 |
| Future Vol, veh/h | 118 | 2 | 10 | 19 | 1 | 13 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 128 | 2 | 11 | 21 | 1 | 14 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int |  |  |  |  |  |  |
| Int Delay, s/veh | 8.1 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations |  |  |  | $\uparrow$ | $\mathbf{A}$ |  |
| Traffic Vol, veh/h | 8 | 168 | 50 | 6 | 9 | 3 |
| Future Vol, veh/h | 8 | 168 | 50 | 6 | 9 | 3 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - None | - | None |  |
| Storage Length | 0 | - | - | - | - | - |
| Ven in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 9 | 183 | 54 | 7 | 10 | 3 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 4.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | M |  |  | $\uparrow$ | $\rightarrow$ |  |
| Traffic Vol, veh/h | 0 | 178 | 81 | 94 | 240 | 0 |
| Future Vol, veh/h | 0 | 178 | 81 | 94 | 240 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 193 | 88 | 102 | 261 | 0 |



| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh | 12.8 |
| Intersection LOS | B |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | \& |  |  | $\uparrow$ | 「 |  | \& |  | \% | $\uparrow$ |  |
| Traffic Vol, veh/h | 1 | 19 | 8 | 113 | 7 | 187 | 4 | 94 | 111 | 252 | 187 | 1 |
| Future Vol, veh/h | 1 | 19 | 8 | 113 | 7 | 187 | 4 | 94 | 111 | 252 | 187 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 21 | 9 | 123 | 8 | 203 | 4 | 102 | 121 | 274 | 203 | 1 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 1 | 1 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 2 |  |  | 1 |  |  | 2 |  |  |  |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 2 |  |  | 1 |  |  | 1 |  |  | 2 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 2 |  |  | 2 |  |  | 1 |  |  |
| HCM Control Delay | 10.5 |  |  | 11.7 |  |  | 12.6 |  |  | 13.9 |  |  |
| HCM LOS | B |  |  | B |  |  | B |  |  | B |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | WBLn2 | SBLn1 | SBLn2 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $2 \%$ | $4 \%$ | $94 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $45 \%$ | $68 \%$ | $6 \%$ | $0 \%$ | $0 \%$ | $99 \%$ |
| Vol Right, \% | $53 \%$ | $29 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 209 | 28 | 120 | 187 | 252 | 188 |
| LT Vol | 4 | 1 | 113 | 0 | 252 | 0 |
| Through Vol | 94 | 19 | 7 | 0 | 0 | 187 |
| RT Vol | 111 | 8 | 0 | 187 | 0 | 1 |
| Lane Flow Rate | 227 | 30 | 130 | 203 | 274 | 204 |
| Geometry Grp | 6 | 6 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.378 | 0.059 | 0.254 | 0.328 | 0.493 | 0.339 |
| Departure Headway (Hd) | 5.983 | 6.947 | 7.001 | 5.812 | 6.476 | 5.966 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 600 | 513 | 513 | 618 | 556 | 603 |
| Service Time | 4.027 | 5.016 | 4.749 | 3.559 | 4.216 | 3.706 |
| HCM Lane V/C Ratio | 0.378 | 0.058 | 0.253 | 0.328 | 0.493 | 0.338 |
| HCM Control Delay | 12.6 | 10.5 | 12.1 | 11.4 | 15.4 | 11.8 |
| HCM Lane LOS | B | B | B | B | C | B |
| HCM 95th-tile Q | 1.8 | 0.2 | 1 | 1.4 | 2.7 | 1.5 |

## 2025 No Build PM




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.3 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | $\uparrow$ | $t$ |  |
| Traffic Vol, veh/h | 2 | 87 | 130 | 12 | 3 | 4 |
| Future Vol, veh/h | 2 | 87 | 130 | 12 | 3 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 95 | 141 | 13 | 3 | 4 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Yr |  |  | -1 | F |  |
| Traffic Vol, veh/h | 2 | 23 | 30 | 135 | 99 | 5 |
| Future Vol, veh/h | 2 | 23 | 30 | 135 | 99 | 5 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 25 | 33 | 147 | 108 | 5 |


| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 324 | 111 | 113 | 0 | - | 0 |
| Stage 1 | 111 | - | - | - | - | - |
| Stage 2 | 213 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | 670 | 942 | 1476 | - | - | - |
| Stage 1 | 914 | - | - | - | - | - |
| Stage 2 | 823 | - | - | - | - | - |
| Platoon blocked, \% |  |  |  | - | - | - |
| Mov Cap-1 Maneuver | 654 | 942 | 1476 | - | - | - |
| Mov Cap-2 Maneuver | 654 | - | - | - | - | - |
| Stage 1 | 892 | - | - | - | - | - |
| Stage 2 | 823 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | NB |  | SB |  |
| HCM Control Delay, s | 9.1 |  | 1.4 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT EBLn1 |  | SBT | SBR |
| Capacity (veh/h) |  | 1476 | - | 910 | - | - |
| HCM Lane V/C Ratio |  | 0.022 | - | 0.03 | - | - |
| HCM Control Delay (s) |  | 7.5 | 0 | 9.1 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0.1 | - | 0.1 | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.8 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Yr |  |  | -1 | a |  |
| Traffic Vol, veh/h | 0 | 98 | 164 | 199 | 134 | 0 |
| Future Vol, veh/h | 0 | 98 | 164 | 199 | 134 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 107 | 178 | 216 | 146 | 0 |



| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh | 13.3 |
| Intersection LOS | B |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | * |  |  | $\uparrow$ | 7 |  | \& |  | ${ }^{\circ}$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 1 | 7 | 4 | 107 | 18 | 243 | 8 | 181 | 109 | 181 | 91 | 1 |
| Future Vol, veh/h | 1 | 7 | 4 | 107 | 18 | 243 | 8 | 181 | 109 | 181 | 91 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 8 | 4 | 116 | 20 | 264 | 9 | 197 | 118 | 197 | 99 | 1 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 1 | 1 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 2 |  |  | 1 |  |  | 2 |  |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 2 |  |  | 1 |  |  | 1 |  |  | 2 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 2 |  |  | 2 |  |  | 1 |  |  |
| HCM Control Delay | 10.2 |  |  | 12.2 |  |  | 15.7 |  |  | 12.3 |  |  |
| HCM LOS | B |  |  | B |  |  | C |  |  | B |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | WBLn2 | SBLn1 | SBLn2 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $3 \%$ | $8 \%$ | $86 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $61 \%$ | $58 \%$ | $14 \%$ | $0 \%$ | $0 \%$ | $99 \%$ |
| Vol Right, \% | $37 \%$ | $33 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 298 | 12 | 125 | 243 | 181 | 92 |
| LT Vol | 8 | 1 | 107 | 0 | 181 | 0 |
| Through Vol | 181 | 7 | 18 | 0 | 0 | 91 |
| RT Vol | 109 | 4 | 0 | 243 | 0 | 1 |
| Lane Flow Rate | 324 | 13 | 136 | 264 | 197 | 100 |
| Geometry Grp | 6 | 6 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.535 | 0.025 | 0.256 | 0.414 | 0.367 | 0.172 |
| Departure Headway (Hd) | 5.951 | 6.953 | 6.788 | 5.643 | 6.712 | 6.196 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 606 | 513 | 529 | 637 | 535 | 578 |
| Service Time | 3.991 | 5.026 | 4.532 | 3.387 | 4.457 | 3.941 |
| HCM Lane V/C Ratio | 0.535 | 0.025 | 0.257 | 0.414 | 0.368 | 0.173 |
| HCM Control Delay | 15.7 | 10.2 | 11.9 | 12.3 | 13.3 | 10.2 |
| HCM Lane LOS | C | B | B | B | B | B |
| HCM 95th-tile Q | 3.2 | 0.1 | 1 | 2 | 1.7 | 0.6 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  | 1 | 4 | a | $\mathbf{F}$ |
| Traffic Vol, veh/h | 118 | 2 | 10 | 19 | 1 | 13 |
| Future Vol, veh/h | 118 | 2 | 10 | 19 | 1 | 13 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 128 | 2 | 11 | 21 | 1 | 14 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int |  |  |  |  |  |  |
| Int Delay, s/veh | 8.1 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations |  |  |  | $\uparrow$ | $\mathbf{A}$ |  |
| Traffic Vol, veh/h | 8 | 168 | 50 | 6 | 9 | 3 |
| Future Vol, veh/h | 8 | 168 | 50 | 6 | 9 | 3 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - None | - | None |  |
| Storage Length | 0 | - | - | - | - | - |
| Ven in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 9 | 183 | 54 | 7 | 10 | 3 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |








## 2025 Antelope Closed PM

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.7 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  | 1 | 个 | a | $\mathbf{7}$ |
| Traffic Vol, veh/h | 25 | 0 | 7 | 54 | 2 | 12 |
| Future Vol, veh/h | 25 | 0 | 7 | 54 | 2 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 27 | 0 | 8 | 59 | 2 | 13 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.3 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | $\uparrow$ | $t$ |  |
| Traffic Vol, veh/h | 2 | 87 | 130 | 12 | 3 | 4 |
| Future Vol, veh/h | 2 | 87 | 130 | 12 | 3 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 95 | 141 | 13 | 3 | 4 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | r |  |  | $\uparrow$ | 个 |  |
| Traffic Vol, veh/h | 2 | 23 | 30 | 135 | 99 | 5 |
| Future Vol, veh/h | 2 | 23 | 30 | 135 | 99 | 5 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 25 | 33 | 147 | 108 | 5 |







| Lane | NBLn1 EBLn1WBLn1 WBLn2 SBLn1 SBLn2 |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $3 \%$ | $8 \%$ | $86 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $61 \%$ | $58 \%$ | $14 \%$ | $0 \%$ | $0 \%$ | $99 \%$ |
| Vol Right, \% | $37 \%$ | $33 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 298 | 12 | 125 | 243 | 181 | 92 |
| LT Vol | 8 | 1 | 107 | 0 | 181 | 0 |
| Through Vol | 181 | 7 | 18 | 0 | 0 | 91 |
| RT Vol | 109 | 4 | 0 | 243 | 0 | 1 |
| Lane Flow Rate | 324 | 13 | 136 | 264 | 197 | 100 |
| Geometry Grp | 6 | 6 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.535 | 0.025 | 0.256 | 0.414 | 0.367 | 0.172 |
| Departure Headway (Hd) | 5.951 | 6.953 | 6.788 | 5.643 | 6.712 | 6.196 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 606 | 513 | 529 | 637 | 535 | 578 |
| Service Time | 3.991 | 5.026 | 4.532 | 3.387 | 4.457 | 3.941 |
| HCM Lane V/C Ratio | 0.535 | 0.025 | 0.257 | 0.414 | 0.368 | 0.173 |
| HCM Control Delay | 15.7 | 10.2 | 11.9 | 12.3 | 13.3 | 10.2 |
| HCM Lane LOS | C | B | B | B | B | B |
| HCM 95th-tile Q | 3.2 | 0.1 | 1 | 2 | 1.7 | 0.6 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  | 1 | 个 | a | $\mathbf{7}$ |
| Traffic Vol, veh/h | 118 | 2 | 10 | 19 | 1 | 13 |
| Future Vol, veh/h | 118 | 2 | 10 | 19 | 1 | 13 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 128 | 2 | 11 | 21 | 1 | 14 |
















| Intersection |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |  |
| Lane Configurations | M |  | \% | 4 | $\uparrow$ |  |  |
| Traffic Vol, veh/h | 2 | 23 | 30 | 135 | 99 | 5 |  |
| Future Vol, veh/h | 2 | 23 | 30 | 135 | 99 | 5 |  |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control | Stop | Stop | Free | Free | Free | Free |  |
| RT Channelized | - | None | - | None | - | None |  |
| Storage Length | 0 | - | 110 | - | - | - |  |
| Veh in Median Storage, \# | \# 0 | - | - | 0 | 0 | - |  |
| Grade, \% | 0 | - | - | 0 | 0 | - |  |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |  |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mvmt Flow | 2 | 25 | 33 | 147 | 108 | 5 |  |





| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh 13 |  |
| Intersection LOS | B |


| Movement EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \& |  |  | $\uparrow$ | 「 | ${ }^{1}$ | F |  | ${ }^{1}$ | $\hat{\dagger}$ |  |
| Traffic Vol, veh/h 1 | 7 | 4 | 107 | 18 | 243 | 8 | 181 | 109 | 181 | 91 | 1 |
| Future Vol, veh/h 1 | 7 | 4 | 107 | 18 | 243 | 8 | 181 | 109 | 181 | 91 | 1 |
| Peak Hour Factor 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow 1 | 8 | 4 | 116 | 20 | 264 | 9 | 197 | 118 | 197 | 99 | 1 |
| Number of Lanes 0 | 1 | 0 | 0 | 1 | 1 | 1 | 1 | 0 | 1 | 1 | 0 |
| Approach EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes 2 |  |  | 1 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach Left SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left 2 |  |  | 2 |  |  | 1 |  |  | 2 |  |  |
| Conflicting Approach RighNB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right 2 |  |  | 2 |  |  | 2 |  |  | 1 |  |  |
| HCM Control Delay 10.2 |  |  | 12.1 |  |  | 14.8 |  |  | 12.3 |  |  |
| HCMLOS B |  |  | B |  |  | B |  |  | B |  |  |


| Lane | NBLn1 NBLn2 EBLn1WBLn1WBLn2 SBLn1 SBLn2 |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $8 \%$ | $86 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $62 \%$ | $58 \%$ | $14 \%$ | $0 \%$ | $0 \%$ | $99 \%$ |
| Vol Right, \% | $0 \%$ | $38 \%$ | $33 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 8 | 290 | 12 | 125 | 243 | 181 | 92 |
| LT Vol | 8 | 0 | 1 | 107 | 0 | 181 | 0 |
| Through Vol | 0 | 181 | 7 | 18 | 0 | 0 | 91 |
| RT Vol | 0 | 109 | 4 | 0 | 243 | 0 | 1 |
| Lane Flow Rate | 9 | 315 | 13 | 136 | 264 | 197 | 100 |
| Geometry Grp | 7 | 7 | 6 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.016 | 0.519 | 0.025 | 0.255 | 0.412 | 0.367 | 0.172 |
| Departure Headway (Hd) | 6.701 | 5.927 | 6.91 | 6.758 | 5.616 | 6.707 | 6.191 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 534 | 608 | 516 | 531 | 639 | 535 | 579 |
| Service Time | 4.446 | 3.671 | 4.984 | 4.504 | 3.362 | 4.453 | 3.937 |
| HCM Lane V/C Ratio | 0.017 | 0.518 | 0.025 | 0.256 | 0.413 | 0.368 | 0.173 |
| HCM Control Delay | 9.6 | 14.9 | 10.2 | 11.8 | 12.3 | 13.3 | 10.2 |
| HCM Lane LOS | A | B | B | B | B | B | B |
| HCM 95th-tile Q | 0 | 3 | 0.1 | 1 | 2 | 1.7 | 0.6 |

## 2025 Sunset Roundabout AM

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  | 1 | 个 | a | $\mathbf{7}$ |
| Traffic Vol, veh/h | 118 | 2 | 10 | 19 | 1 | 13 |
| Future Vol, veh/h | 118 | 2 | 10 | 19 | 1 | 13 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 128 | 2 | 11 | 21 | 1 | 14 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int |  |  |  |  |  |  |
| Int Delay, s/veh | 8.1 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations |  |  |  | $\uparrow$ | $\mathbf{A}$ |  |
| Traffic Vol, veh/h | 8 | 168 | 50 | 6 | 9 | 3 |
| Future Vol, veh/h | 8 | 168 | 50 | 6 | 9 | 3 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - None | - | None |  |
| Storage Length | 0 | - | - | - | - | - |
| Ven in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 9 | 183 | 54 | 7 | 10 | 3 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |







## 2025 Sunset Roundabout PM

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.7 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{\beta}$ |  |  | 4 | 1 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 25 | 0 | 7 | 54 | 2 | 12 |
| Future Vol, veh/h | 25 | 0 | 7 | 54 | 2 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 27 | 0 | 8 | 59 | 2 | 13 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.3 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | r |  |  | -1 | 个 |  |
| Traffic Vol, veh/h | 2 | 87 | 130 | 12 | 3 | 4 |
| Future Vol, veh/h | 2 | 87 | 130 | 12 | 3 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 95 | 141 | 13 | 3 | 4 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Yr |  |  | -1 | F |  |
| Traffic Vol, veh/h | 2 | 23 | 30 | 135 | 99 | 5 |
| Future Vol, veh/h | 2 | 23 | 30 | 135 | 99 | 5 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 25 | 33 | 147 | 108 | 5 |






| Lane | NBLn1 EBLn1WBLn1 WBLn2 SBLn1 SBLn2 |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $3 \%$ | $8 \%$ | $86 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $61 \%$ | $58 \%$ | $14 \%$ | $0 \%$ | $0 \%$ | $99 \%$ |
| Vol Right, \% | $37 \%$ | $33 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 298 | 12 | 125 | 243 | 181 | 92 |
| LT Vol | 8 | 1 | 107 | 0 | 181 | 0 |
| Through Vol | 181 | 7 | 18 | 0 | 0 | 91 |
| RT Vol | 109 | 4 | 0 | 243 | 0 | 1 |
| Lane Flow Rate | 324 | 13 | 136 | 264 | 197 | 100 |
| Geometry Grp | 6 | 6 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.535 | 0.025 | 0.256 | 0.414 | 0.367 | 0.172 |
| Departure Headway (Hd) | 5.951 | 6.953 | 6.788 | 5.643 | 6.712 | 6.196 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 606 | 513 | 529 | 637 | 535 | 578 |
| Service Time | 3.991 | 5.026 | 4.532 | 3.387 | 4.457 | 3.941 |
| HCM Lane V/C Ratio | 0.535 | 0.025 | 0.257 | 0.414 | 0.368 | 0.173 |
| HCM Control Delay | 15.7 | 10.2 | 11.9 | 12.3 | 13.3 | 10.2 |
| HCM Lane LOS | C | B | B | B | B | B |
| HCM 95th-tile Q | 3.2 | 0.1 | 1 | 2 | 1.7 | 0.6 |

Intersection Failure Years AM

| Intersection |  |  |  |
| :--- | ---: | ---: | ---: |
| Intersection Delay, s/veh | 14.6 |  |  |
| Intersection LOS | B |  | NB |
| Approach | EB | 1 | SB |
| Entry Lanes | 1 | 1 | 1 |
| Conflicting Circle Lanes | 1 | 1 |  |
| Adj Approach Flow, veh/h | 588 | 59 | 694 |
| Demand Flow Rate, veh/h | 600 | 483 | 606 |
| Vehicles Circulating, veh/h | 597 | 3 | 255 |
| Vehicles Exiting, veh/h | 264 | 1194 | 231 |
| Ped Vol Crossing Leg, \#/h | 0 | 0 | 0 |
| Ped Cap Adj | 1.000 | 1.000 | 1.000 |
| Approach Delay, s/veh | 25.4 | 5.9 | 10.8 |
| Approach LOS | D | A | B |


| Lane | Left | Left | Left |
| :--- | ---: | ---: | ---: |
| Designated Moves | LR | LT | TR |
| Assumed Moves | LR | LT | TR |
| RT Channelized |  |  |  |
| Lane Util | 1.000 | 1.000 | 1.000 |
| Follow-Up Headway, s | 2.609 | 2.609 | 4.609 |
| Critical Headway, s | 4.976 | 4.976 | 606 |
| Entry Flow, veh/h | 600 | 483 | 1064 |
| Cap Entry Lane, veh/h | 751 | 1376 | 0.981 |
| Entry HV Adj Factor | 0.980 | 0.980 | 594 |
| Flow Entry, veh/h | 588 | 474 | 1043 |
| Cap Entry, veh/h | 736 | 1349 | 0.570 |
| V/C Ratio | 0.799 | 0.351 | 10.8 |
| Control Delay, s/veh | 25.4 | 5.9 | B |
| LOS | D | A | 4 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.7 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{F}$ |  |  | 4 | i | $\mathbf{7}$ |
| Traffic Vol, veh/h | 389 | 0 | 109 | 840 | 31 | 187 |
| Future Vol, veh/h | 389 | 0 | 109 | 840 | 31 | 187 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | 100 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 423 | 0 | 118 | 913 | 34 | 203 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 15.4 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | -1 | $\mathbf{7}$ |  |
| Traffic Vol, veh/h | 14 | 593 | 887 | 82 | 20 | 27 |
| Future Vol, veh/h | 14 | 593 | 887 | 82 | 20 | 27 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 6 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 15 | 645 | 964 | 89 | 22 | 29 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3.1 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | * |  |  | $\uparrow$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 11 | 124 | 162 | 729 | 535 | 27 |
| Future Vol, veh/h | 11 | 124 | 162 | 729 | 535 | 27 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | \# 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 12 | 135 | 176 | 792 | 582 | 29 |



| Intersection |  |  |  |
| :--- | ---: | ---: | ---: |
| Intersection Delay, s/veh | 20.3 |  |  |
| Intersection LOS | C |  | SB |
| Approach | 1 | 1 | 1 |
| Entry Lanes | 1 | 1 | 1 |
| Conflicting Circle Lanes | 348 | 405 |  |
| Adj Approach Flow, veh/h | 355 | 1225 | 413 |
| Demand Flow Rate, veh/h | 403 | 689 | 563 |
| Vehicles Circulating, veh/h | 696 | 0 | 0 |
| Vehicles Exiting, veh/h | 0 | 758 | 1.000 |
| Ped Vol Crossing Leg, \#/h | 1.000 | 0 | 16.1 |
| Ped Cap Adj | 8.5 | 1.000 | C |


| Lane | Left | Left | Left |
| :--- | ---: | ---: | ---: |
| Designated Moves | LR | LT | TR |
| Assumed Moves | LR | LT |  |
| RT Channelized |  |  |  |
| Lane Util | 1.000 | 1.000 | 1.000 |
| Follow-Up Headway, s | 2.609 | 2.609 | 4.609 |
| Critical Headway, s | 4.976 | 4.976 | 413 |
| Entry Flow, veh/h | 355 | 1249 | 685 |
| Cap Entry Lane, veh/h | 915 | 1380 | 0.981 |
| Entry HV Adj Factor | 0.980 | 0.981 | 405 |
| Flow Entry, veh/h | 348 | 1225 | 672 |
| Cap Entry, veh/h | 897 | 1353 | 0.603 |
| V/C Ratio | 0.388 | 0.905 | 16.1 |
| Control Delay, s/veh | 8.5 | 25.1 | C |
| LOS | A | D | 4 |

Intersection
Intersection Delay, s/veh18.3
Intersection LOS $\quad$ C

| Movement EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | 4 |  |  | $\uparrow$ | F |  | * |  | ${ }^{1}$ | t |  |
| Traffic Vol, veh/h 1 | 9 | 5 | 134 | 23 | 304 | 10 | 226 | 136 | 226 | 114 | 1 |
| Future Vol, veh/h 1 | 9 | 5 | 134 | 23 | 304 | 10 | 226 | 136 | 226 | 114 | 1 |
| Peak Hour Factor 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow 1 | 10 | 5 | 146 | 25 | 330 | 11 | 246 | 148 | 246 | 124 | 1 |
| Number of Lanes 0 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 1 | 1 | 0 |
| Approach EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes 2 |  |  | 1 |  |  | 2 |  |  | 1 |  |  |
| Conflicting Approach Left SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left 2 |  |  | 1 |  |  | 1 |  |  | 2 |  |  |
| Conflicting Approach RighNB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right 1 |  |  | 2 |  |  | 2 |  |  | 1 |  |  |
| HCM Control Delay 11.3 |  |  | 15.7 |  |  | 24.7 |  |  | 15.2 |  |  |
| HCMLOS B |  |  | C |  |  | C |  |  | C |  |  |


| Lane | NBLn1 EBLn1WBLn1WBLn2 SBLn1 SBLn2 |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $3 \%$ | $7 \%$ | $85 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $61 \%$ | $60 \%$ | $15 \%$ | $0 \%$ | $0 \%$ | $99 \%$ |
| Vol Right, \% | $37 \%$ | $33 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 372 | 15 | 157 | 304 | 226 | 115 |
| LT Vol | 10 | 1 | 134 | 0 | 226 | 0 |
| Through Vol | 226 | 9 | 23 | 0 | 0 | 114 |
| RT Vol | 136 | 5 | 0 | 304 | 0 | 1 |
| Lane Flow Rate | 404 | 16 | 171 | 330 | 246 | 125 |
| Geometry Grp | 6 | 6 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.723 | 0.036 | 0.346 | 0.565 | 0.497 | 0.235 |
| Departure Headway (Hd) | 6.437 | 8.008 | 7.304 | 6.154 | 7.28 | 6.763 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 558 | 450 | 490 | 582 | 491 | 527 |
| Service Time | 4.51 | 6.008 | 5.085 | 3.934 | 5.065 | 4.548 |
| HCM Lane V/C Ratio | 0.724 | 0.036 | 0.349 | 0.567 | 0.501 | 0.237 |
| HCM Control Delay | 24.7 | 11.3 | 13.9 | 16.7 | 17.1 | 11.6 |
| HCM Lane LOS | C | B | B | C | C | B |
| HCM 95th-tile Q | 6 | 0.1 | 1.5 | 3.5 | 2.7 | 0.9 |

## Appendix D. Construction Cost Estimates

## QUANTITIES ARE BASED ON THE CONCEPT ENGINEERING PLANS

TOTALS AND UNIT PRICES ARE CALCULATED IN PRESENT WORTH OR PRESENT VALUE DOLLARS. ADJUSTMENTS SHOULD BE MADE FOR YEARS BEYOND THE PRESENT YEAR TO BETTER ESTIMATE NEEDED CAPITAL DOLLARS FOR A FUTURE CAPITAL IMPROVEMENT PLAN(S).

Does not include Engineering Design Fees but may be covered under contingency

Does not include ROW or utility relocaitons but may be covered under contingency

Assumes full resurfacing for any partial roadway work




| ITEM | CATEGORY \& ITEM DESCRIPTION | UNIT | PRICE | QUANTITY | AMOUNT |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Category 1 - Preliminary |  |  |  |  |
|  | 20\% of Categories 2, 4, 5, 6 |  |  | ORY TOTAL | \$61,344 |
|  | Category 2 - Grading |  |  |  |  |
| 201 | Class 1 excavation | CY | \$30.00 | 0.00 | \$0 |
| 202 | Removal of existing pavement | CY | \$35.00 | 645.00 | \$22,575 |
| 203 | Removal of existing sidewalk | CY ${ }^{\text {¢ }}$ \$100.00 $\quad$ CATEGORY TOTAL |  |  | \$ |
|  |  |  |  |  | \$22,575 |
|  | Category 3 - Drainage |  |  |  |  |
|  | $5 \%$ of Categories 2, 4, 5, 6 |  |  | ORY TOTAL | \$15,336 |
|  | Category 4-Structures |  |  |  |  |
| 401 | Park bench | EA \$ 1,000.00 ${ }^{\text {CATEGORY TOTAL }}$ |  |  | S0 |
|  |  |  |  |  | \$0 |
|  | Category 5 - Paving |  |  |  |  |
| 501 | 5 inch white reflective thermoplastic pavement markings | LF | \$1.50 | 4,045.00 | \$6,068 |
| 502 | 5 inch yellow reflective thermoplastic pavement markings | LF | \$1.50 | 3,500.00 | \$5,250 |
| 503 | 10 inch white reflective thermoplastic pavement markings | LF | \$1.75 | 0.00 |  |
| 504 | 10 inch yellow reflective thermoplastic pavement markings | LF | \$1.75 | 0.00 | \$0 |
| 505 | 12 inch white reflective thermoplastic pavement markings | LF | \$2.00 | 960.00 | \$1,920 |
| 506 | 24 inch white reflective thermoplastic pavement markings | LF | \$7.00 | 1,215.00 | \$8,505 |
| 507 | Preformed thermoplastic pavement marking legend and arrows | SF | \$25.00 | 0.00 | \$0 |
| 508 | 2 inch superpave asphalt mix for surface | TON | \$80.00 | 0.00 | \$0 |
| 509 | 6 inch superpave asphalt mix for base | TON | \$160.00 | 0.00 | \$0 |
| 510 | 4 inch graded aggregate base course | SY $\quad$ \$6.00 CATEGORY TOTAL |  |  | \$0 |
|  |  |  |  |  | \$21,743 |
|  | Category 6 - Shoulders |  |  |  |  |
| 601 | ADA ramp | EA | \$1,250.00 | 15.00 | \$18,750 |
| 602 | Brick walkway | SF | \$10.00 | 16,240.00 | \$162,400 |
| 606 | Type A curb and gutter - 12 inch gutter pan 8 inch depth | LF | \$25.00 | 3,250.00 | \$81,250 |
| 607 | 5 inch concrete sidewalk | SF $\quad$ \$7.00 $\quad$ CATEGORY TOTAL |  |  | \$0 |
|  |  |  |  |  | \$262,400 |
|  | Category 7 - Landscaping |  |  |  |  |
|  | 10\% of Categories 2, 4, 5, 6 | CATEGORY TOTAL |  |  | \$30,672 |
|  | Category 8 - Traffic |  |  |  |  |
| 801 | Lighting structure | EA | \$13,000.00 | 0.00 | \$0 |
| 802 | Relocate existing telecom box | EA | \$500.00 | 0.00 | \$0 |
| 803 | Square perforated tubular steel sign post | EA | \$100.00 | 38.00 | \$3,800 |
| 804 | Square perforated tubular steel anchor bases | EA | \$100.00 | 38.00 | \$3,800 |
| 805 | Sheet aluminum signs | SF | \$50.00 | 275.00 | \$13,750 |
| 806 | Relocate existing ground mounted signs | SF | \$35.00 | 0.00 | \$0 |
|  |  |  |  |  | \$0 |
| CATEGORY TOTAL |  |  |  |  | \$21,350 |
|  |  |  |  |  |  |
|  |  |  |  | SUB TOTAL | \$435,419 |
|  |  |  |  |  |  |
|  |  |  | CONTINGENCY | 25\% | \$108,855 |
|  |  | RIGHT-OF-WAY/EASEMENTS $\quad$ N/A |  |  |  |
|  |  |  |  |  | \$544,273 |
|  |  |  |  |  |  |
|  |  | CONSTRUCTION TOTAL |  |  | \$550,000 |


${ }^{1} \$ 20 /$ SF for commercial property and $\$ 5 / \mathrm{SF}$ for residential land assumed.




## Appendix E. Survey Results and Stakeholder Comments



From: Jack Moore [jmcfd86@me.com](mailto:jmcfd86@me.com)
Sent: Tuesday, January 26, 2021 6:25 PM
To: Jeremy Yates [jyates@casperwy.gov](mailto:jyates@casperwy.gov)
Subject: Bar Nunn Salt Creek Feedback

CAUTION: This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.

Jeremy,

Howdy, hope this email finds you well. I wanted to give you my very brief feedback on the Bar Nunn study. I was only able to attend online. These are the three options, that in my opinion, will have the most significant positive impact to the Town of Bar Nunn. These three options will drastically increase the safety and access to Bar Nunn while providing some fiscal savings.

1) Abandon the predetermined section of Antelope

- As noted in the presentation there is a positive fiscal impact associated with this option.
- Most importantly however, I foresee no significant safety impact to the Town of Bar Nunn. Ambulance and Fire Protection Response to Bar Nunn come from Wyoming Medical Center and Natrona County Fire Protection District primarily. The slight increase in response time to the neighborhoods effected by abandoning the small section of Antelope and rerouting of response through Sunset Blvd would most likely be negligible. The affected areas I speak of would be houses on Antelope St. south of Palomino as well as the entirety of Palomino St. I would estimate less than one min. impact to response times.

2) Improvement and Widening (including turn lanes and walking path) of Salt Creek from McMurry Blvd to Howard street.

- This option by far poses the highest financial impact to the area, but the positive impact to the Town of Bar Nunn would be easily measured in Safety, Access and Aesthetics.
- Safe access for foot traffic and bike traffic by constructing a pathway to Howard from the Town of Bar Nunn will make Bar Nunn a much more desirable location to live. The Loaf and Jug will continue to be a stop gap for those trips to the store for small items that don't require a run all the way into Casper. Adults and young adults
would chose to utilize their bikes or even get some exercise. Over the years I have wanted to ride my bike into Casper to report to work, I could never bring myself to do it do to the lack of a shoulder on the road. This is a huge safety issue that would be the best possible option of the three I am suggesting.

3) Round about at Sunset.

- This one is the extra nicety I would like to see. This will slow traffic and create a gateway to our wonderfull community. We need that aesthetic focal point to advertise I presence.
- Also, round about will slow traffic and enhance the already safer Salt Creek corridor after the widening and addition of turn lanes.

Thank you for your hard work on this project. I would definitely vote for these three items should they show up on a ballot.

Jack L. Moore
5318 Nez Perce Tr
Bar Nunn, WY 82601
------Original Message-----
From: Mary Sue Sorenson < marysuesorenson@townofbarnunn.com>
Sent: Monday, February 1, 2021 10:08 PM
To: Jeremy Yates < jyates@casperwy.gov>
Cc: marysuesorenson@townofbarnunn.com
Subject: Bar Nunn Salt Creek Hwy Corridor Traffic Study \& Plan

CAUTION: This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.

Jeremy:
I hope it is not too late to add my comments.

NO Build Antelope - Is the road really that bad?
Concept 1 - Left Turn Lanes - this is the best option in my opinion Concept 2 - Sunset Roundabout - I am not a fan of roundabouts.
Concept 3 -Channelization- I am not in favor of adding stop signs on Antelope at Sunset or at Prairie Concept 4-Sunset Streetscape - not a necessary expenditure for the Town Concept 5 -Truck Access Road - is this really needed?
Concept 6 - Way too expensive!
Concept 7 - SC Side Path - am I missing something here because why would anyone want to walk/bike along Salt Creek Hwy when we have such nice paths in the Town?

Thanks for your efforts on this project. Is another public meeting being planned for the future?

Mary Sue Sorenson
Bar Nunn Town Council
970.846.7254

Appendix F: Roundabout Design Guidelines

SOURCE: FEDERAL HIGHWAY ADMINISTRATION

## Modern Roundabout Design

 Elements

## Roundabout Design Elements

- Modern Roundabouts differ from rotaries and traffic circles which have higher speeds, stop or signal control at entry points, little or no deflection and active uses in the center island.



## Single Lane Roundabouts

- This type of roundabout is characterized as having a single lane entry at all legs and one circulatory lane, with a generous inscribed circle diameters and more tangential entries and exits, resulting in higher vehicle capacities. Their design allows slightly higher speeds at the entry, on the circulatory roadway, and at the exit.
- The roundabout design is focused on achieving consistent entering and circulating vehicle speeds.
- The geometric design includes raised splitter islands, a non-mountable central island, The geometric design includes raised splitter islands that incorporate at-grade pedestrian storage areas, and a non-mountable central island. There is usually an apron surrounding the non-mountable part of the
 compact central island to accommodate large vehicles.


## Roundabout Advantages



## Example Designs

- Lacy Road, WI (left)
- Golden Gate Park, CA (bottom)
- Folly Quarter Road, Ellicott City, MD



[^0]:    *Note: Approaches with free movements and no vehicle delay are omitted

